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**GENERAL NOTES:**

- ALL LINEAR DIMENSIONS ARE IN MILLIMETERS UNLESS NOTED OTHERWISE (UNO).
- ALL DIMENSIONS AND CONDITIONS SHALL BE VERIFIED IN THE FIELD BY THE CONTRACTOR AND ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE CONTRACTING OFFICER BEFORE PROCEEDING WITH THE AFFECTED PORTION OF THE WORK.
- CONTRACTOR SHALL INFORM THE CONTRACTING OFFICER OF ALL DISCREPANCIES BETWEEN DRAWINGS OF DIFFERENT TRADES, PRIOR TO INITIATION OF ANY WORK.
- CONTRACTOR SHALL IMMEDIATELY NOTIFY THE CONTRACTING OFFICER WHEN, IN THE COURSE OF CONSTRUCTION, CONDITIONS ARE UNCOVERED WHICH ARE UNANTICIPATED OR OTHERWISE APPEAR TO PRESENT A DANGEROUS CONDITION.
- STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH CIVIL SHOP DRAWINGS.
- SECTIONS AND DETAILS SHOWN ARE TO BE CONSIDERED TYPICAL FOR ALL SIMILAR CONDITIONS.
- WORK NOT INDICATED AS PART OF THE DRAWINGS BUT REASONABLY IMPLIED TO BE SIMILAR TO THAT SHOWN AT CORRESPONDING LOCATIONS SHALL BE INCLUDED.

**DESIGN LOADS:**

ALL LOADS ARE IN ACCORDANCE WITH TIA-222-G UNO.

- DEAD LOADS: TOWERS, ACCESSORIES, AND ATTACHMENTS
- LIVE LOADS: NOT APPLICABLE
- WIND LOADS: BASIC WIND SPEED: 145 KM/H (WITHOUT ICE)  
64.4 KM/H (WITH ICE)  
DEFLECTION CALCULATIONS: 96.6 KM/H  
STRUCTURE CLASS: III  
EXPOSURE CATEGORY: C  
TOPOGRAPHIC CATEGORY: 1  
CREST HEIGHT: 0.00 M
- ICE LOADS: ICE DENSITY: 897 KG/CM  
ICE THICKNESS: 50.8 MM (NOMINAL)  
ICE THICKNESS CONSIDERED TO INCREASE WITH HEIGHT

SEISMIC DESIGN: SEISMIC SPECTRAL RESPONSE BASED ON  $S_s=1.25$  AND  $S_1=0.60$ , SITE CLASS D, IMPORTANCE FACTOR  $I = 1.00$ . IN ACCORDANCE WITH ASCE 7-05. COMBINATION OF ORTHOGONAL SEISMIC LOADS IS NOT REQUIRED FOR NON-BUILDING STRUCTURES PER ASCE 7-05, SECTION 12.5, AND IS NOT REQUIRED BY TIA-222-G. THEREFORE TRANSVERSE AND LONGITUDINAL EARTHQUAKE LOADING ARE CONSIDERED SEPARATELY AND INDEPENDENTLY.

- EQUIPMENT LOADS ARE BASED ON THE TABLE TITLED "EQUIPMENT LOADS."

**FOUNDATIONS:**

- FOUNDATIONS HAVE BEEN DESIGNED FOR THE FOLLOWING ASSUMED GEOTECHNICAL PARAMETERS:  
ALLOWABLE BEARING CAPACITY = 1.0 kg/cm<sup>2</sup> [2000 PSF]  
MODULUS OF SUBGRADE REACTION = 27.145 MPa/m [100 PSI/IN]  
 $K_a = 0.33$   
 $K_p = 3.0$   
 $K_o = 0.50$
- THE CONTRACTOR SHALL VERIFY ACTUAL SUBSURFACE CONDITIONS MEET ASSUMED GEOTECHNICAL DESIGN PARAMETERS.
- ALL FOOTINGS SHALL BE PLACED ON NATURAL UNDISTURBED SOIL OR ON COMPACTED SELECT GRANULAR MATERIAL FILL PREPARED AS FOLLOWS:  
(A) REMOVE UNSUITABLE MATERIAL BELOW THE FOOTING AND REPLACE WITH COMPACTED SELECT GRANULAR MATERIAL FILL TO A DEPTH WHERE NATURAL SOIL AND OR COMPACTED FILL IS ENCOUNTERED.  
(B) FILL MATERIAL MUST BE PLACED IN LIFTS UP TO A MINIMUM OF 150mm THICKNESS. EACH LIFT SHALL BE COMPACTED TO A MINIMUM OF 95% OF THE MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D1557 AT MOISTURE CONTENT WITHIN MINUS 1% TO PLUS 2% OF THE OPTIMUM.
- ALL DIMENSIONS, ELEVATIONS AND EXISTING CONDITIONS SHALL BE VERIFIED IN THE FIELD
- BEARING SOIL SHALL BE INSPECTED AND APPROVED BY THE QUALITY CONTROL ENGINEER BEFORE CONSTRUCTING ANY FOOTINGS. NO FOUNDATION CONCRETE SHALL BE PLACED IN WATER OR ON FROZEN SUBGRADE MATERIAL.

**ABBREVIATIONS AND SYMBOLS**

ADD'L	ADDITIONAL	FIN. GR.	FINISHED GRADE	T&B	TOP AND BOTTOM
ALT.	ALTERNATE	FTG	FOOTING	T.O.	TOP OF
ARCH	ARCHITECTURAL	INFO	INFORMATION	T.O.S.	TOP OF STEEL
BOT.	BOTTOM	JT.	JOINT	TYP.	TYPICAL
B.O.	BOTTOM OF	LLH	LONG LEG HORIZONTAL	U.N.O.	UNLESS NOTED OTHERWISE
BSMT	BASEMENT	LLV	LONG LEG VERTICAL	VERT.	VERTICAL
CLR	CLEAR	MFR.	MANUFACTURER	VIF	VERIFY IN FIELD
CONT.	CONTINUOUS	MAX.	MAXIMUM	WP	WORKING POINT
COL	COLUMN	MIN.	MINIMUM		
COORD.	COORDINATE	N-S	NORTH-SOUTH		
CTR.	CENTER	NO.	NUMBER		
DEMO	DEMOLITION	O.C.	ON CENTER		
DIA.	DIAMETER	O.D.	OUTSIDE DIAMETER		
DN	DOWN	OP'G	OPENING		
DWG	DRAWING	PL	PLATE		
E-W	EAST-WEST	PROP.	PROPOSED		
E.F.	EACH FACE	PSF	POUNDS PER SQUARE FOOT		
EL.	ELEVATION	REINF.	REINFORCING		
EOD	EDGE OF DECK	SIM	SIMILAR		
EOS	EDGE OF SLAB	SQ.	SQUARE		
EQ.	EQUAL	SS	STAINLESS STEEL	PL	PLATE
E.W.	EACH WAY	STD	STANDARD	CL	CENTERLINE
EXIST	EXISTING	STIFF	STIFFENER		
FDN	FOUNDATION	STRUCT	STRUCTURAL	DO	DITTO

**FOUNDATIONS (CONT.):**

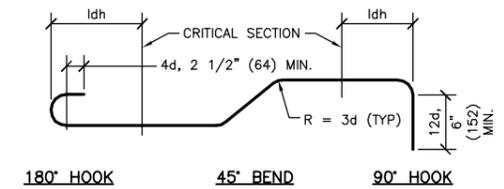
- IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO PROVIDE TEMPORARY SUPPORT AS NECESSARY DURING EXCAVATION AND UNDERPINNING TO MAINTAIN THE INTEGRITY OF ANY ADJACENT EXISTING STRUCTURES AND/OR INFRASTRUCTURE.
- CONTRACTOR SHALL IMMEDIATELY NOTIFY THE CONTRACTING OFFICER IF THE EXISTING SOIL CONDITIONS DO NOT MEET THE MINIMUM REQUIREMENTS, AND IF LOADING CRITERIA DIFFERS FROM WHAT IS PRESENTED HEREIN.

**CAST IN PLACE CONCRETE:**

- CONCRETE WORK SHALL CONFORM TO:  
ACI 301-05 - SPECIFICATIONS FOR STRUCTURAL CONCRETE.  
ACI 318-08 - BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND COMMENTARY.
- CONCRETE SHALL HAVE A MINIMUM CONCRETE COMPRESSIVE STRENGTH AT 28 DAYS,  $f'_c = 28 \text{ MPa}$  (4000 PSI). THE MAXIMUM WATER-CEMENT RATIO OF 0.45 (BY WEIGHT). ALL CONCRETE EXPOSED TO WEATHER SHALL BE AIR-ENTRAINED.
- CONCRETE SHALL BE CONTROLLED NORMAL WEIGHT CONCRETE, PROPORTIONED, MIXED AND PLACED UNDER THE SUPERVISION OF AN APPROVED QUALITY CONTROL ENGINEER.
- THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCING:  
(A) CONCRETE PLACED AGAINST THE EARTH 75mm  
(B) SIDES OF FOOTINGS, & PEDESTALS  
18  $\phi$  BAR AND LARGER 50mm  
16  $\phi$  BAR AND SMALLER 40mm  
(C) BEAMS AND COLUMNS 40mm  
(D) ELEVATED SLABS, WALLS, AND SLABS-ON-GRADE (FROM TOP) 40mm
- CHAMFER EXPOSED EDGES 20mm U.N.O.
- ALL REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615, GRADE 60. SEE SPLICE TABLE FOR LAP LENGTHS. MINIMUM YIELD STRENGTH  $F_y = 4218 \text{ kg/cm}^2$
- DO NOT WELD OR BEND REINFORCEMENT IN FIELD UNLESS SPECIFICALLY SHOWN OR APPROVED BY ENGINEER.
- REINFORCING BARS EXTEND 12 BAR DIAMETERS BUT NOT LESS THAN 300mm BEYOND BEND U.N.O.
- NO BARS SHALL BE CUT OR OMITTED IN THE FIELD FOR CONDUIT. BARS MAY BE MOVED ASIDE WITHOUT CHANGE IN LEVEL WITH THE APPROVAL OF THE QUALITY CONTROL ENGINEER.
- REINFORCEMENT STEEL SHALL BE CONTINUOUS THROUGH ALL CONSTRUCTION JOINTS. ALL CONSTRUCTION JOINTS SHALL BE KEYS U.N.O.
- WHERE VERTICAL CONSTRUCTION JOINTS ARE NOT SHOWN, OR WHEN ALTERNATE LOCATIONS ARE PROPOSED, DRAWINGS SHOWING LOCATION OF CONSTRUCTION JOINTS AND CONCRETE PLACING SEQUENCE SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO PREPARATION OF THE REINFORCEMENT SHOP DRAWINGS. CONCRETE SHALL BE PLACED WITHOUT HORIZONTAL CONSTRUCTION JOINTS U.N.O.
- ALL KEYS SHALL BE 50mm BY 100mm NOMINAL U.N.O.
- DETAILING, FABRICATION, AND ERECTION OF REINFORCEMENT SHALL CONFORM TO ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, ACI 315 DETAILS AND DETAILING OF CONCRETE REINFORCEMENT, AND CRSI MANUAL OF STANDARD PRACTICE.
- CONTRACTOR SHALL COORDINATE LOCATIONS OF ELECTRICAL CONDUITS, GROUNDS, SLEEVES, INSERTS, ETC. WITH CONCRETE CONSTRUCTION. NO PIPES SHALL PASS THROUGH CONCRETE WITHOUT THE PERMISSION OF THE CONTRACTING OFFICER. STEEL PIPE SLEEVES SHALL BE PROVIDED AND SPACED A MINIMUM OF THREE PIPE DIAMETERS ON CENTER. CONDUIT AND OTHER EMBEDDED ITEMS SHALL BE CLEAN AND FREE OF OIL AND OTHER FOREIGN MATTER SUCH AS LOOSE COATINGS OR RUST, PAINT AND SCALE.
- PROVIDE ALL NECESSARY CHAIRS, CHAIR BARS, SPACERS, ETC., WIRED SECURELY TO HOLD REINFORCEMENT IN POSITION. THESE ACCESSORIES SHALL BE PLASTIC BOOTED WHERE CONCRETE IS TO BE EXPOSED TO WEATHER OR MOISTURE. WIRE TIES SHALL BE 1.5 mm  $\phi$  OR HEAVIER BLACK ANNEALED STEEL WIRE.
- THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS SHOWING REINFORCING STEEL PLACEMENT, SCHEDULES, SIZES, GRADES, AND SPLICING AND BENDING DETAILS. DRAWINGS SHALL SHOW SUPPORT DETAILS INCLUDING TYPES, SIZES AND SPACING.
- REINFORCEMENT SHALL BE STORED OFF THE GROUND ON PLATFORMS, SKIDS OR OTHER SUPPORTS.

**LEGEND (FOR REINFORCING SHOWN IN PLAN OR ELEVATION)**

	90° HOOK IN THE PLANE OF THE DRAWING
	90° BEND PERPENDICULAR TO THE PLANE OF THE DRAWING
	HOOK PERPENDICULAR TO THE PLANE OF THE DRAWING
	180° HOOK IN THE PLANE OF THE DRAWING
	OFFSET IN THE PLANE OF THE DRAWING
<b>BAR SIZE</b>	$\phi 10$ $\phi 12$ $\phi 16$ $\phi 20$ $\phi 25$
<b>ldh</b>	190   230   305   380   475



**180° HOOK      45° BEND      90° HOOK**

NOTE: "d" = BAR DIAMETER.

**BENDS IN REINFORCING BARS N.T.S.**

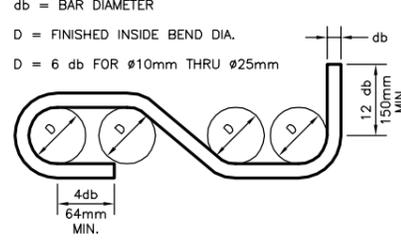
MINIMUM RE-BAR SPLICE LENGTHS mm\*  
 $f_y = 60,000 \text{ psi}$     $f'_c = 4,000 \text{ psi}$

BAR SIZE $\phi \text{mm}$	TOP BARS*	OTHER BARS
10	650	500
12	815	635
16	1016	788
20	1220	940
25	2032	1550

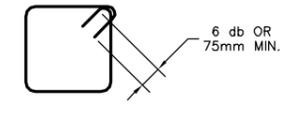
**SPLICE NOTES**

- BASED ON NORMAL WEIGHT CONCRETE, UNCOATED BARS, CLEAR SPACING NOT LESS THAN FOUR BAR DIAMETERS, AND CLEAR COVER NOT LESS THAN 40mm.
- WHERE SPACING BETWEEN BARS IS LESS THAN FOUR BAR DIAMETERS, OR CLEAR COVER IS LESS THAN TWO BAR DIAMETERS, INCREASE SPLICE LENGTHS SHOWN BY 50%.
- \*\* HORIZONTAL BARS WITH MORE THAN 300mm OF CONCRETE CAST BELOW THE BARS AS DEFINED BY A.C.I. 318. WHERE HORIZONTAL WALL REINFORCEMENT IS UNIFORMLY SPACED IN A VERTICAL PLANE AT 300mm MAXIMUM SPACING, LENGTHS MAY BE AS FOR "OTHER BARS".

**BEND DIAMETER SCHEDULE**



**STANDARD HOOKS AND BENDS**  
WALL BARS SHALL BE TIED TOGETHER



**PEDESTAL TIES**  
SPLICES FOR SUCCESSIVE TIES TO BE PLACED AT ALTERNATE CORNERS

**TYPICAL REINFORCING BAR STIRRUPS AND TIES N.T.S.**

UNLESS OTHERWISE NOTED, ALL DIMENSIONS SHOWN ARE IN MILLIMETERS.

PROTOTYPE DESIGN SUBMITTAL

DATE	08/15/10
DESIGNED BY:	FPH
DRAWN BY:	WAS
CHECKED BY:	FPH
DATE	08/15/10
DESIGNED BY:	TETRA TECH
DRAWN BY:	WAS
CHECKED BY:	FPH
FILE NO.:	TWR00-S-001

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TETRA TECH

USAID THREE TOWERS  
PROTOTYPE DESIGN, AFGHANISTAN

GENERAL NOTES  
SHEET 1 OF 2

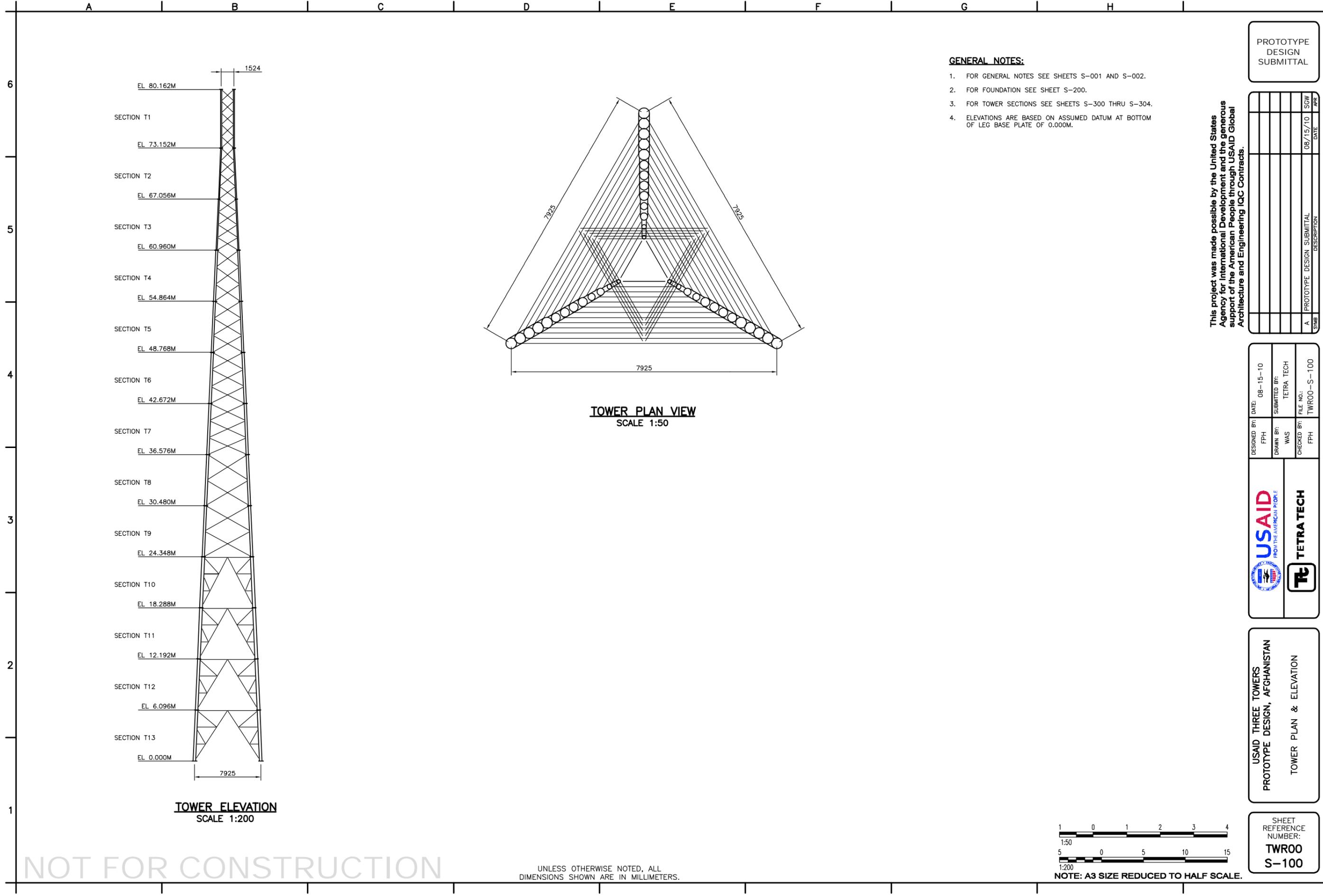
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**TWR00 S-001**

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NOTE: A3 SIZE REDUCED TO HALF SCALE.



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- GENERAL NOTES:**
1. FOR GENERAL NOTES SEE SHEETS S-001 AND S-002.
  2. FOR FOUNDATION SEE SHEET S-200.
  3. FOR TOWER SECTIONS SEE SHEETS S-300 THRU S-304.
  4. ELEVATIONS ARE BASED ON ASSUMED DATUM AT BOTTOM OF LEG BASE PLATE OF 0.000M.

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SYMB	DESCRIPTION	DATE	SCW	APP
A	PROTOTYPE DESIGN SUBMITTAL	08/15/10		

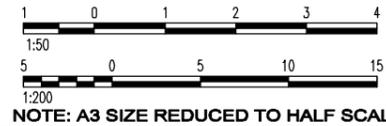
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TOWER PLAN & ELEVATION

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**TWR00  
S-100**

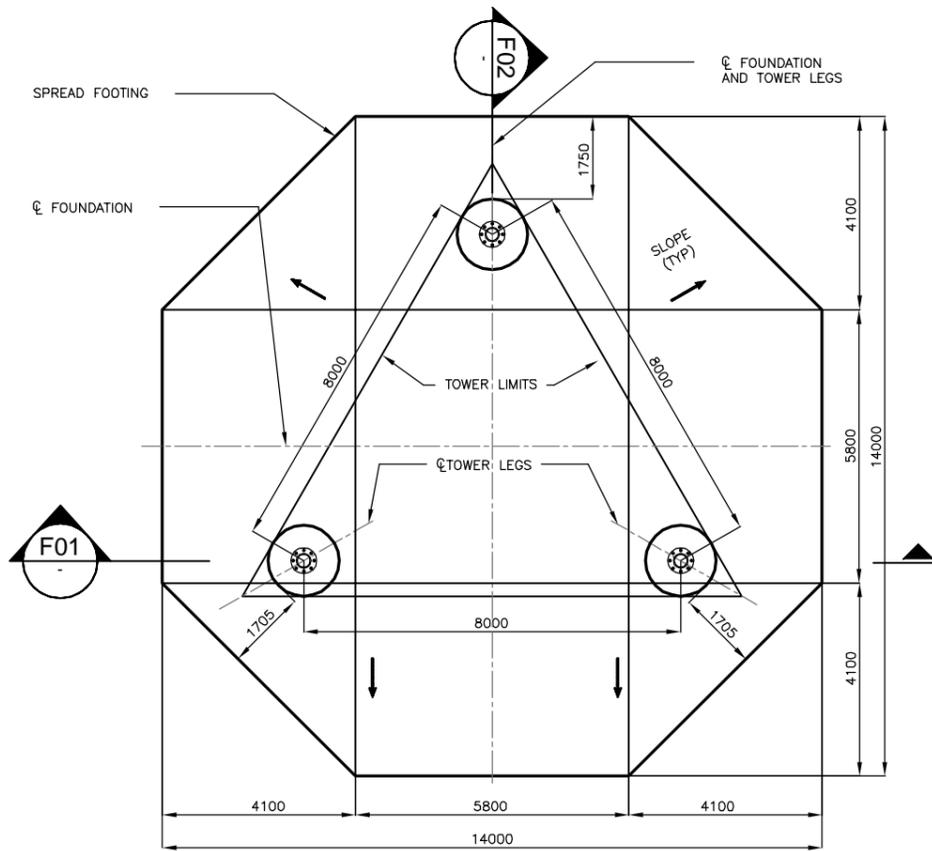


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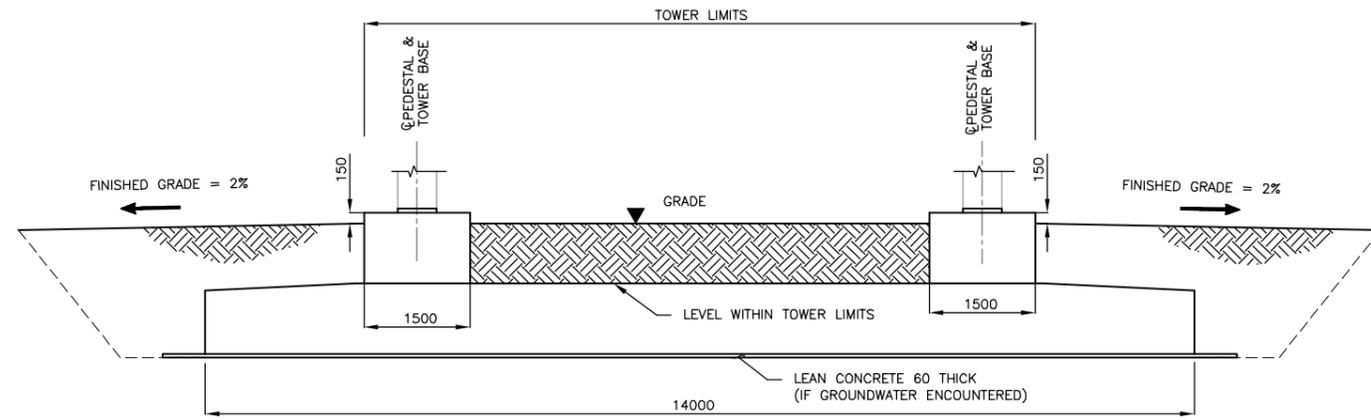
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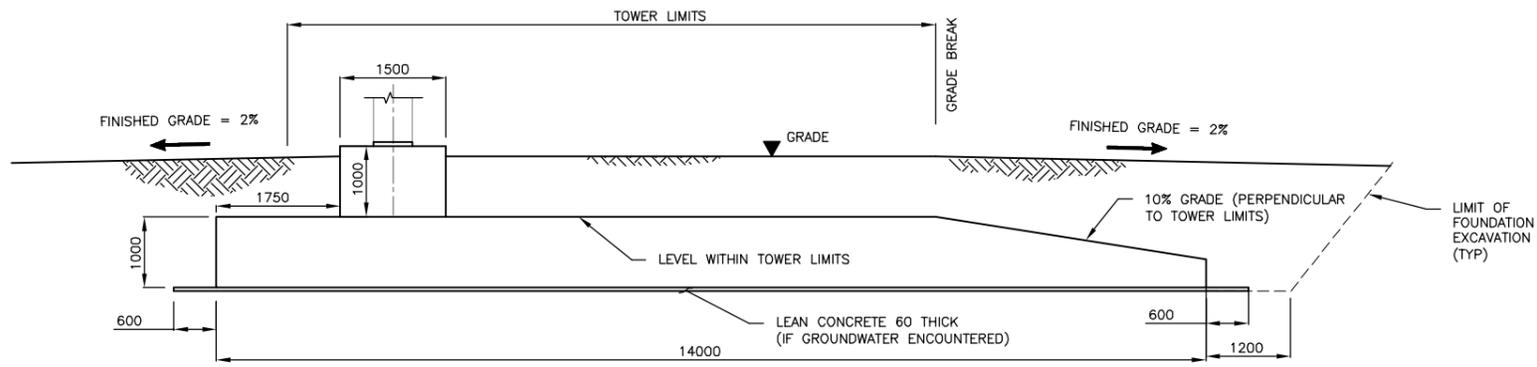
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**TOWER FOUNDATION PLAN VIEW**  
SCALE 1:75



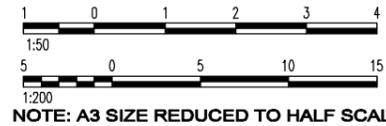
**TOWER FOUNDATION SECTION F01**  
SCALE 1:50



**TOWER FOUNDATION SECTION F02**  
SCALE 1:50

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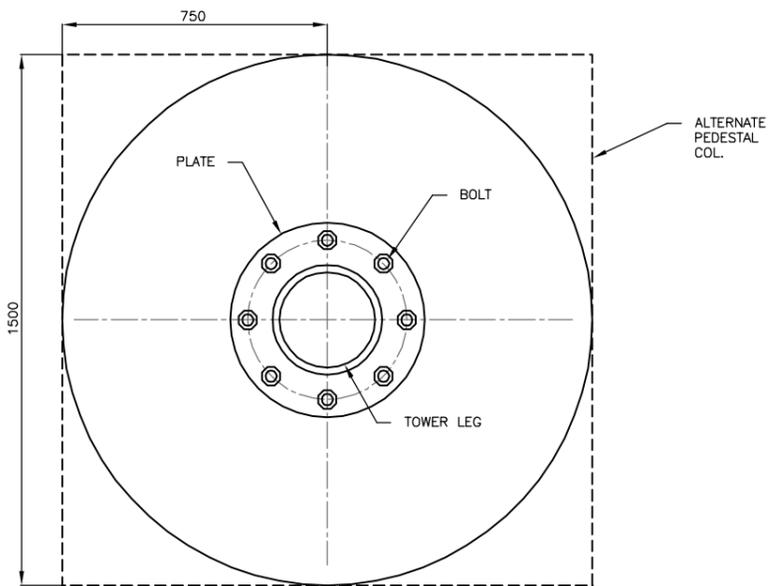
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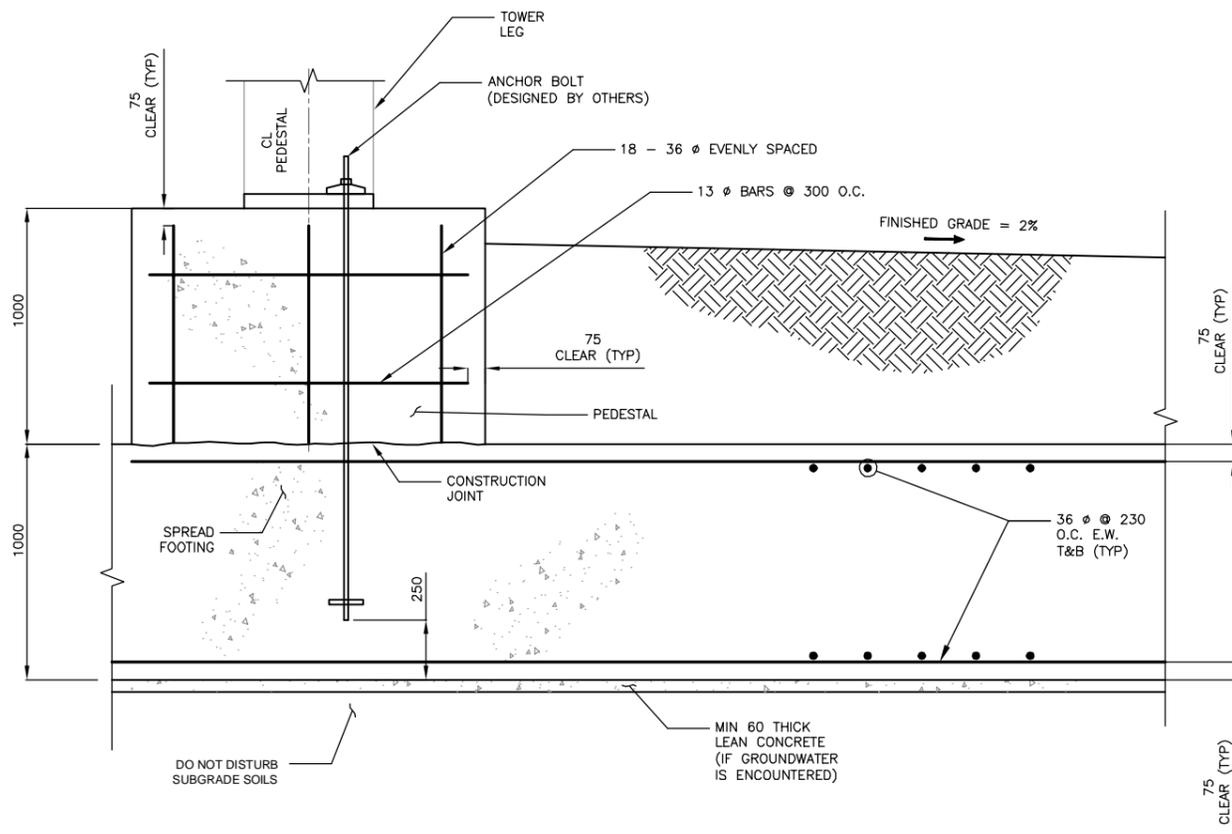
USAID THREE TOWERS  
PROTOTYPE DESIGN, AFGHANISTAN  
TOWER FOUNDATION PLAN & ELEVATION  
SHEET 1 OF 2

SHEET REFERENCE NUMBER:  
TWR00  
S-200

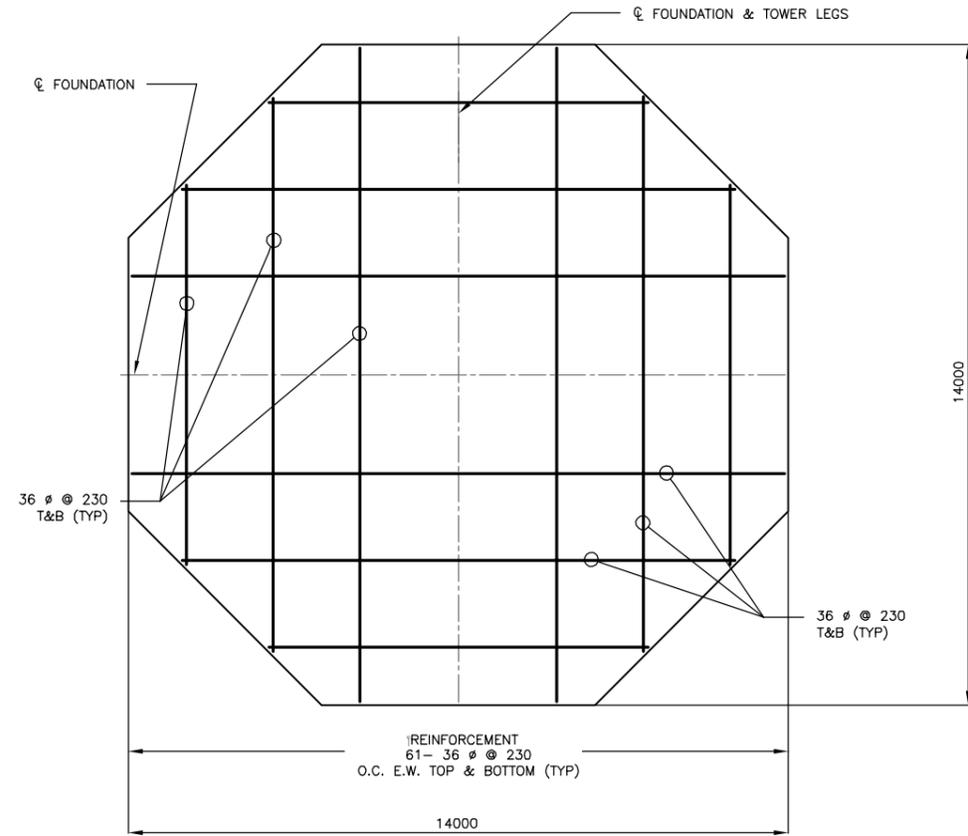
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**TYPICAL PEDESTAL TOWER BASE PLAN VIEW**  
SCALE 1:10



**PEDESTAL AND SPREAD FOOTING REINFORCEMENT DETAIL**  
SCALE 1:15



**SPREAD FOOTING REINFORCEMENT PLAN DETAIL**  
SCALE 1:75

**GENERAL NOTES:**

1. ALL CONCRETE WORK SHALL CONFORM TO ACI-301-05 AND ACI-318-08.
2. SPREAD FOOTING SHALL BE PLACED ON NATURAL UNDISTURBED SOIL.
3. THE SPREAD FOOTING SHALL BEAR ON FIRM TO HARD UNIFORM MATERIALS AT UNIFORM ELEVATION. SUBGRADE MUST BE APPROVED BY GEOTECHNICAL ENGINEER.
4. MINIMUM 28-DAY CONCRETE COMPRESSIVE STRENGTH FOR SPREAD FOOTING: 281 kg/cm<sup>2</sup> (4000 psi).
5. MINIMUM 28-DAY CONCRETE COMPRESSIVE STRENGTH FOR PEDESTAL: 281 kg/cm<sup>2</sup> (4000 psi).
6. CONCRETE SHALL BE AIR-ENTRAINED.
7. 36 mm BAR = #11 IMPERIAL, 13 mm BAR = #4 IMPERIAL
8. MINIMUM REBAR SPLICE LENGTH = 2000.
9. MINIMUM CLEARANCE ALL REBAR = 75.
10. MINIMUM YIELD STRENGTH OF REINFORCING BAR = 4218 kg/cm<sup>2</sup> (60 ksi).
11. MINIMUM SOIL BEARING PRESSURE = 1.0 kg/cm<sup>2</sup> (2,000 psf)
12. FILL MATERIAL MUST BE PLACED IN 150 LIFTS AND COMPACTED TO WITHIN 95% MAXIMUM DRY DENSITY AND BE WITHIN PLUS OR MINUS 2% OF OPTIMUM MOISTURE CONTENT AS DETERMINED BY ASTM D-1557.



NOTE: A3 SIZE REDUCED TO HALF SCALE.

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0	PROTOTYPE DESIGN SUBMITTAL	08/15/10		

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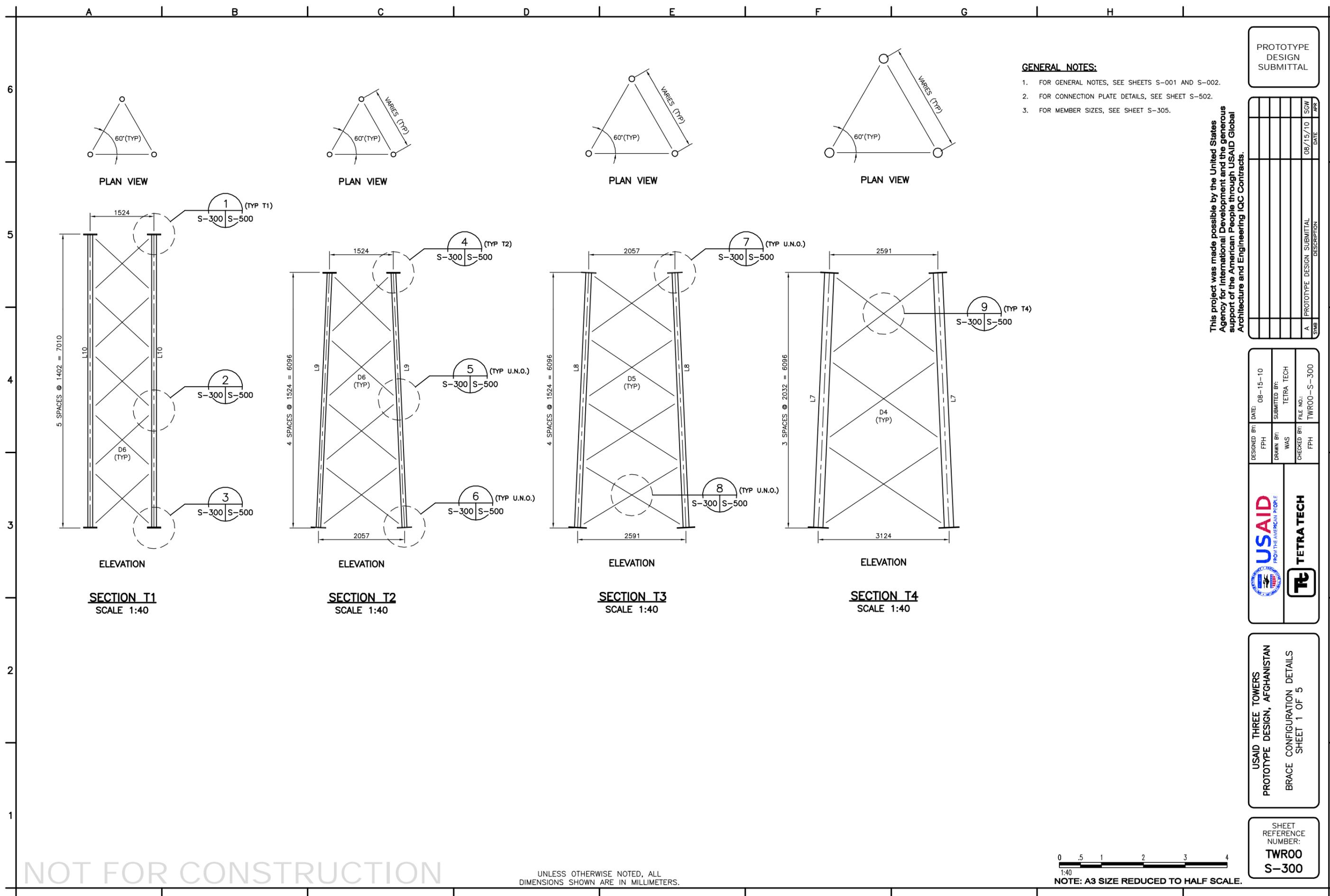
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PROTOTYPE DESIGN, AFGHANISTAN  
TOWER FOUNDATION PLAN & ELEVATION  
SHEET 2 OF 2

SHEET  
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S-201**

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- GENERAL NOTES:**
- FOR GENERAL NOTES, SEE SHEETS S-001 AND S-002.
  - FOR CONNECTION PLATE DETAILS, SEE SHEET S-502.
  - FOR MEMBER SIZES, SEE SHEET S-305.

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A	PROTOTYPE DESIGN SUBMITTAL	08/15/10	SCW

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FPH	08-15-10
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WAS	TETRA TECH
CHECKED BY:	FILE NO.:
FPH	TWR00-S-300

USAID THREE TOWERS  
 PROTOTYPE DESIGN, AFGHANISTAN  
 BRACE CONFIGURATION DETAILS  
 SHEET 1 OF 5

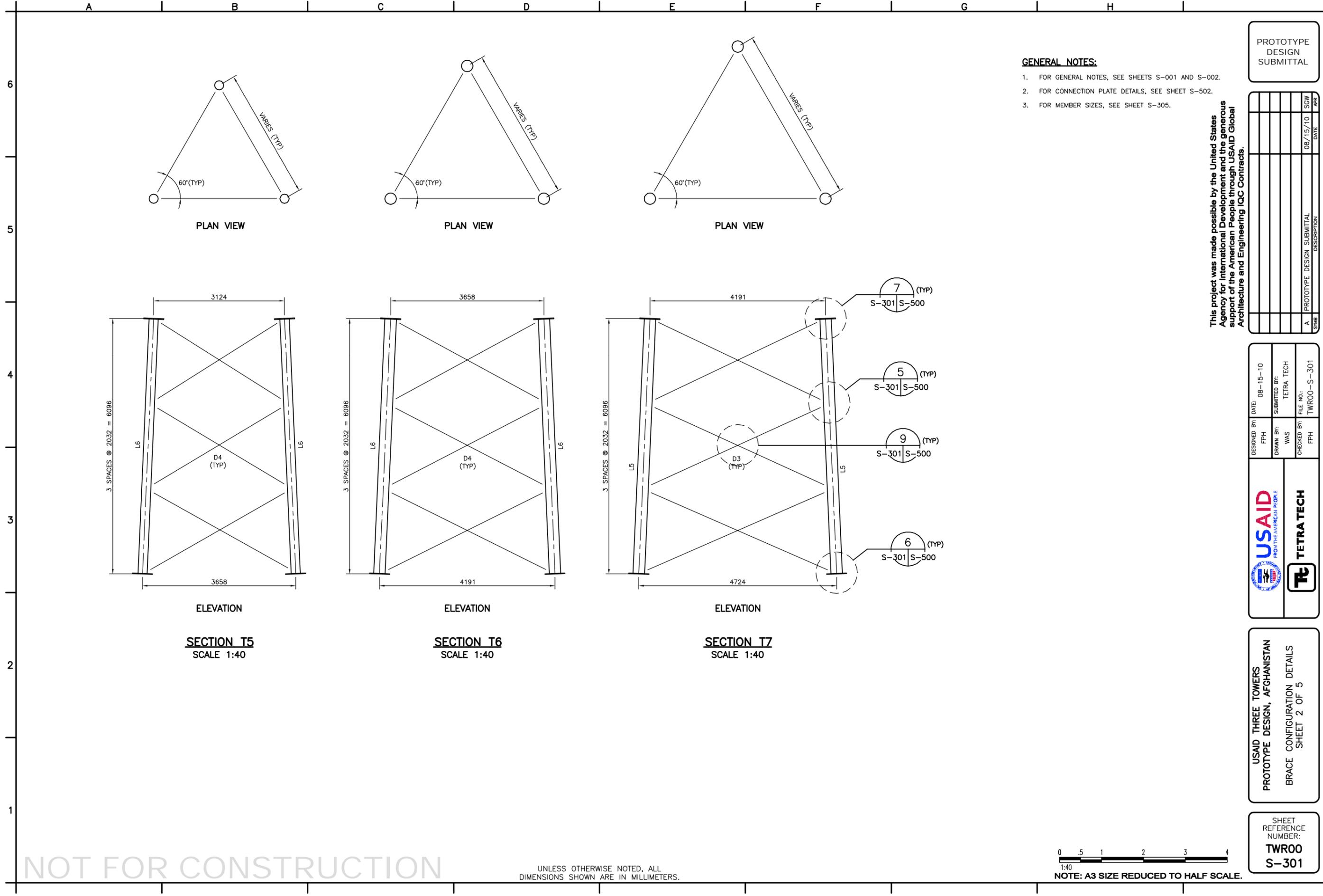
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**GENERAL NOTES:**

1. FOR GENERAL NOTES, SEE SHEETS S-001 AND S-002.
2. FOR CONNECTION PLATE DETAILS, SEE SHEET S-502.
3. FOR MEMBER SIZES, SEE SHEET S-305.

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A	PROTOTYPE DESIGN SUBMITTAL	08/15/10	SCW

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PROTOTYPE DESIGN, AFGHANISTAN  
BRACE CONFIGURATION DETAILS  
SHEET 2 OF 5

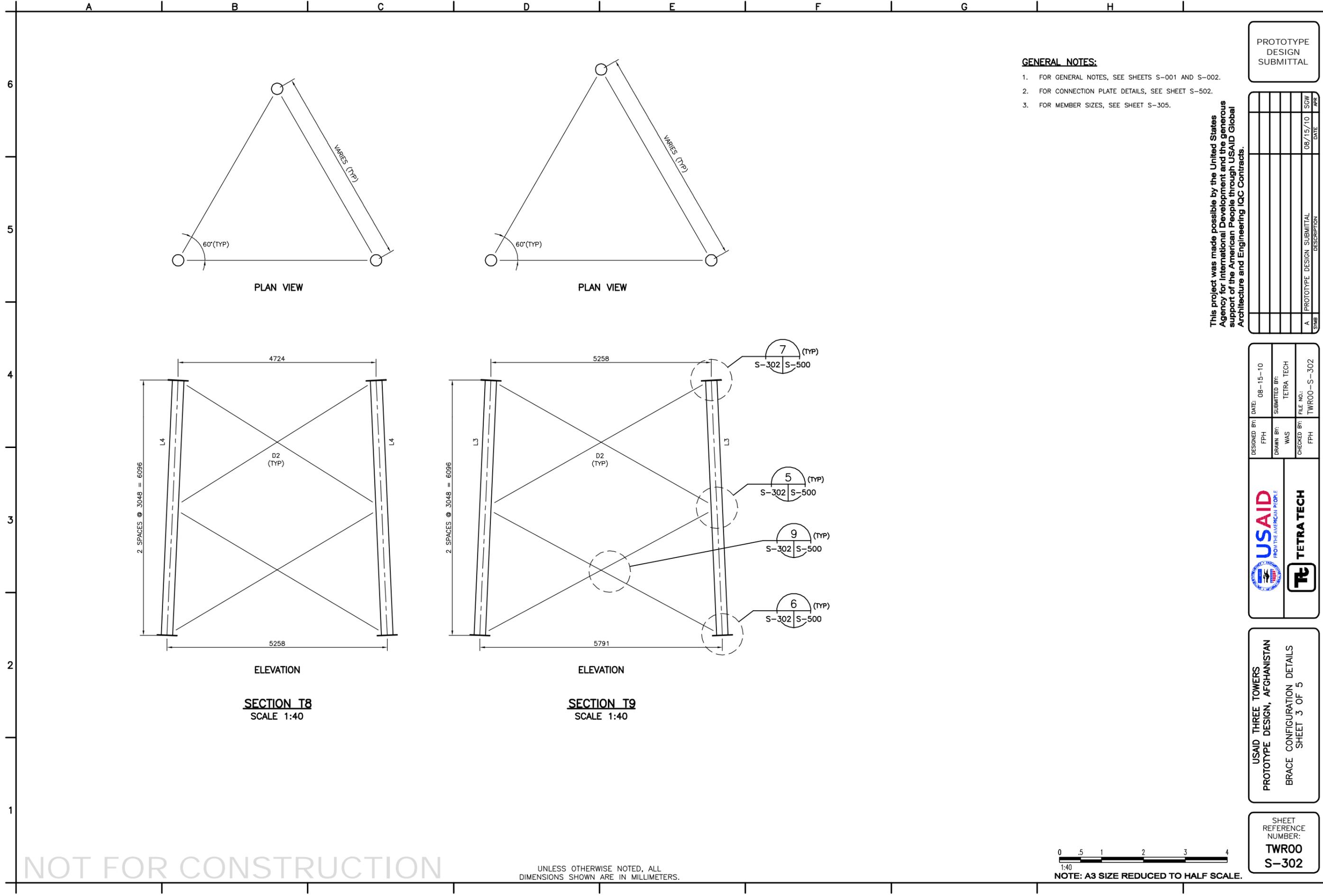
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NUMBER:  
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S-301**

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**GENERAL NOTES:**

- FOR GENERAL NOTES, SEE SHEETS S-001 AND S-002.
- FOR CONNECTION PLATE DETAILS, SEE SHEET S-502.
- FOR MEMBER SIZES, SEE SHEET S-305.

PROTOTYPE  
DESIGN  
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USAID THREE TOWERS  
PROTOTYPE DESIGN, AFGHANISTAN  
BRACE CONFIGURATION DETAILS  
SHEET 3 OF 5

SHEET  
REFERENCE  
NUMBER:  
**TWR00  
S-302**

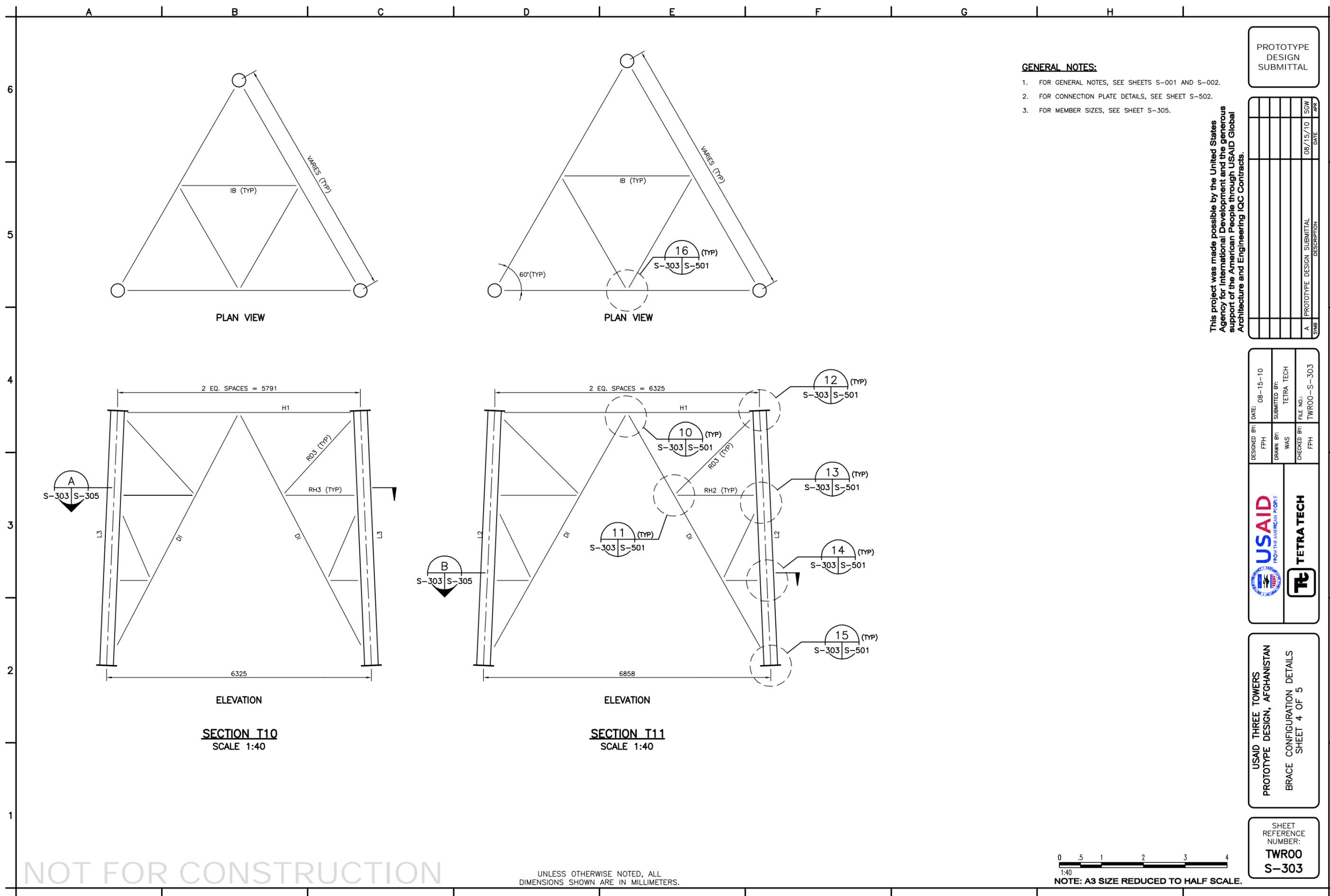
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NOTE: A3 SIZE REDUCED TO HALF SCALE.

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**GENERAL NOTES:**

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2. FOR CONNECTION PLATE DETAILS, SEE SHEET S-502.
3. FOR MEMBER SIZES, SEE SHEET S-305.

PROTOTYPE  
DESIGN  
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A	PROTOTYPE DESIGN SUBMITTAL	08/15/10		

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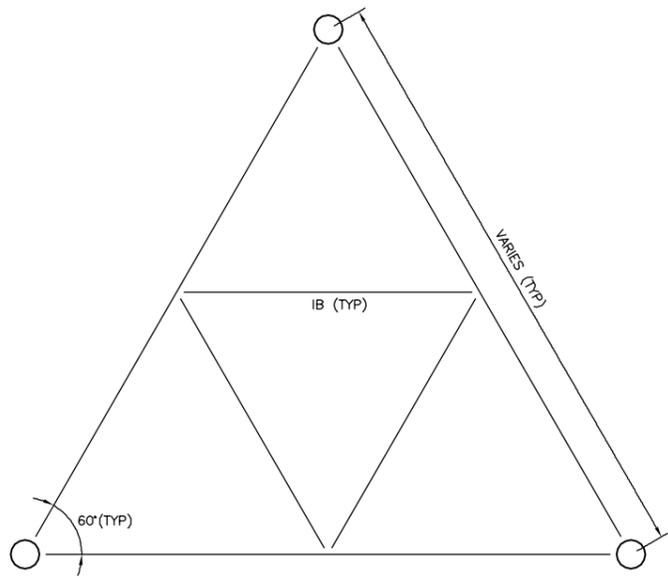
USAID THREE TOWERS  
PROTOTYPE DESIGN, AFGHANISTAN  
BRACE CONFIGURATION DETAILS  
SHEET 4 OF 5

SHEET  
REFERENCE  
NUMBER:  
**TWR00  
S-303**

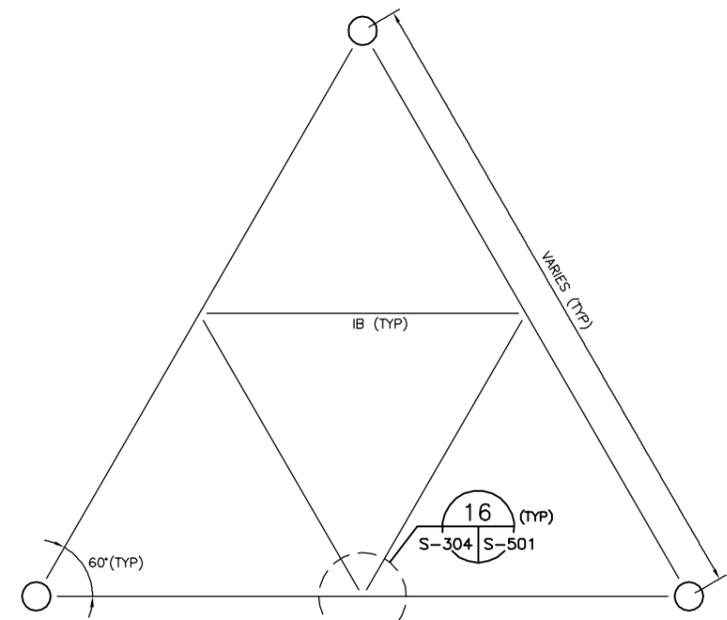
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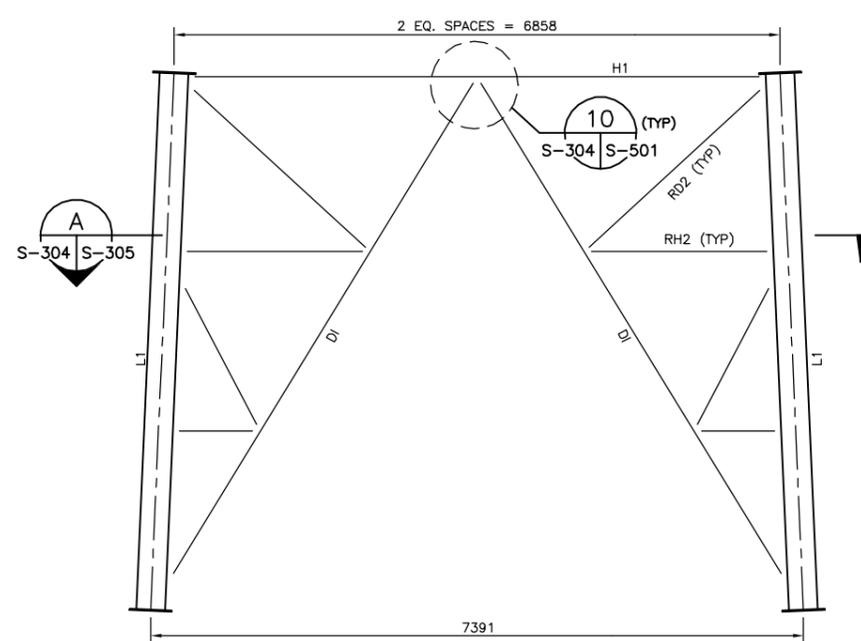




PLAN VIEW

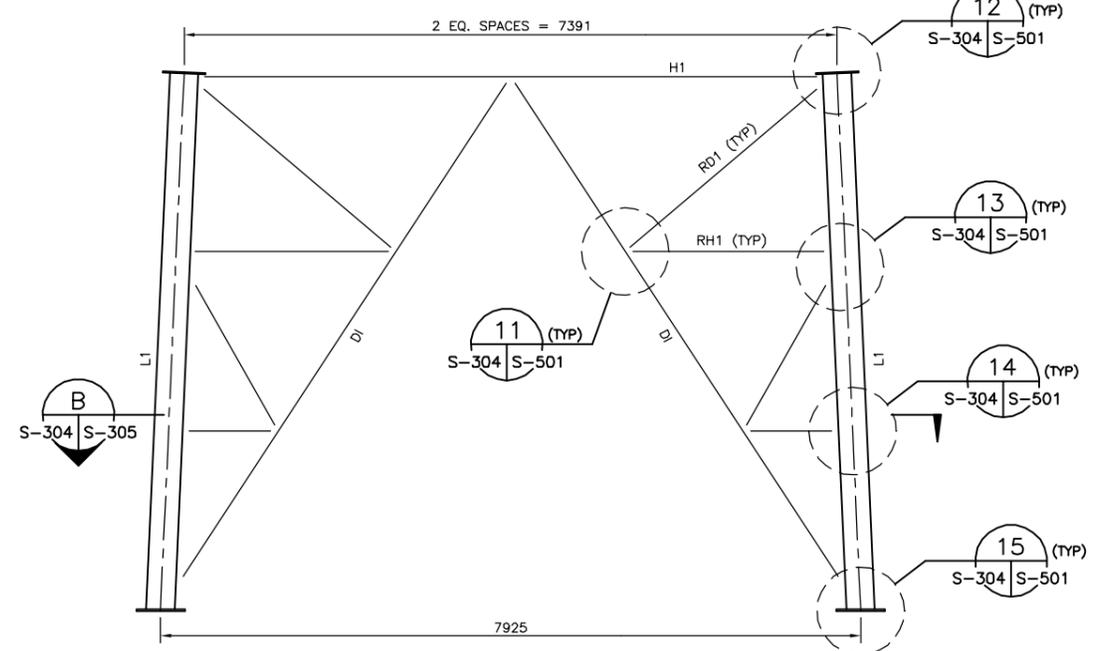


PLAN VIEW



ELEVATION

SECTION T12  
SCALE 1:40



ELEVATION

SECTION T13  
SCALE 1:40

**GENERAL NOTES:**

1. FOR GENERAL NOTES, SEE SHEETS S-001 AND S-002.
2. FOR CONNECTION PLATE DETAILS, SEE SHEET S-502.
3. FOR MEMBER SIZES, SEE SHEET S-305.

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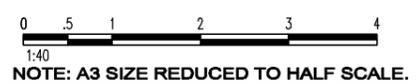
PROTOTYPE  
DESIGN  
SUBMITTAL

SYMB	DESCRIPTION	DATE	SCW	APR
A	PROTOTYPE DESIGN SUBMITTAL	08/15/10		

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FPH	TWR00-S-304

USAID THREE TOWERS  
 PROTOTYPE DESIGN, AFGHANISTAN  
 BRACE CONFIGURATION DETAILS  
 SHEET 5 OF 5

SHEET  
 REFERENCE  
 NUMBER:  
**TWR00  
 S-304**

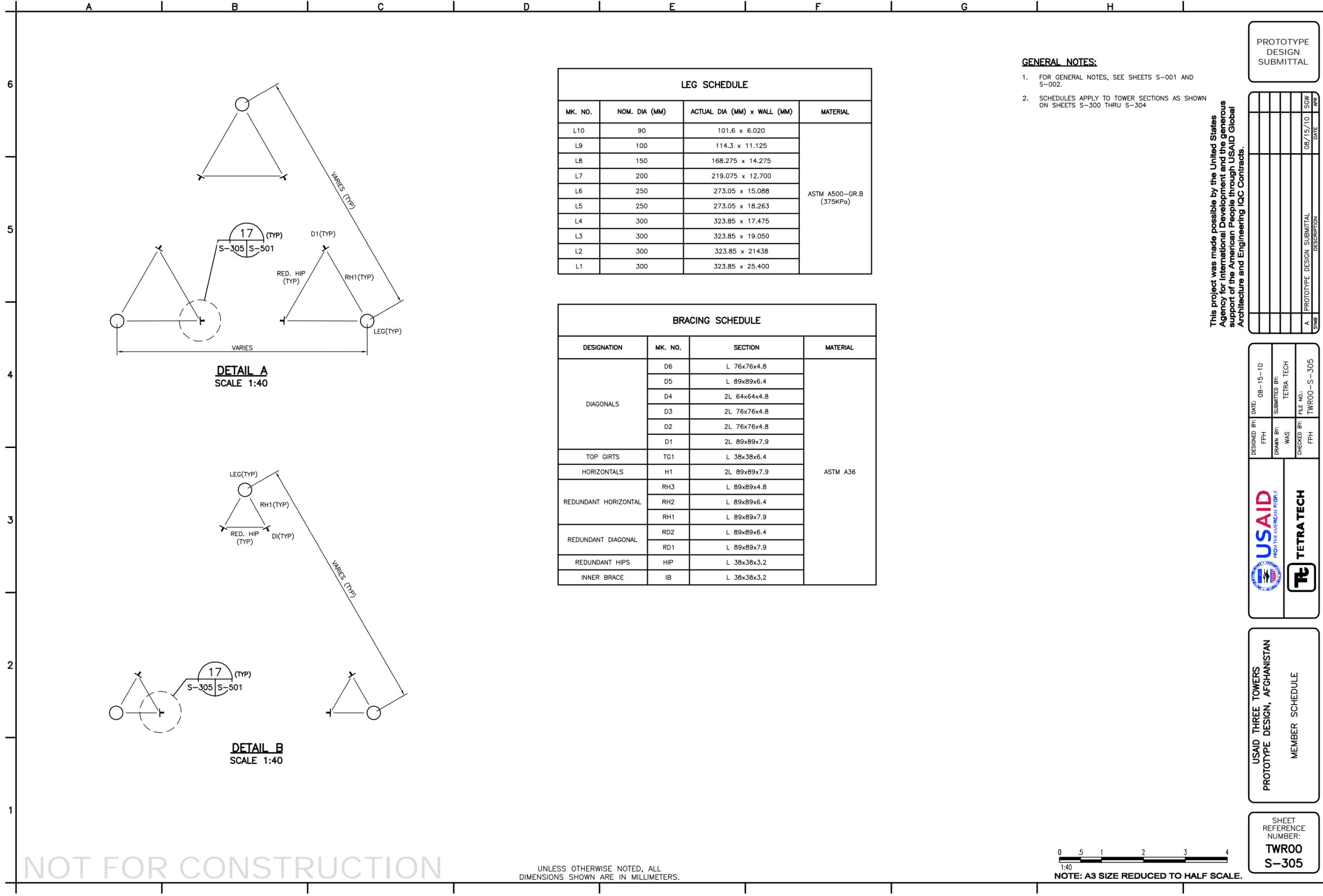


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**DETAIL A**  
SCALE 1:40

**DETAIL B**  
SCALE 1:40

**LEG SCHEDULE**

MK. NO.	NOM. DIA (MM)	ACTUAL DIA (MM) x WALL (MM)	MATERIAL
L10	90	101.6 x 6.020	ASTM A500-GR.B (375KPa)
L9	100	114.3 x 11.125	
L8	150	168.275 x 14.275	
L7	200	219.075 x 12.700	
L6	250	273.05 x 15.088	
L5	250	273.05 x 18.263	
L4	300	323.85 x 17.475	
L3	300	323.85 x 19.050	
L2	300	323.85 x 21.438	
L1	300	323.85 x 25.400	

**BRACING SCHEDULE**

DESIGNATION	MK. NO.	SECTION	MATERIAL
DIAGONALS	D6	L 76x76x4.8	ASTM A36
	D5	L 89x89x6.4	
	D4	2L 64x64x4.8	
	D3	2L 76x76x4.8	
	D2	2L 76x76x4.8	
	D1	2L 89x89x7.9	
TOP GIRTS	TG1	L 38x38x6.4	
HORIZONTALS	H1	2L 89x89x7.9	
REDUNDANT HORIZONTAL	RH3	L 89x89x4.8	
	RH2	L 89x89x6.4	
	RH1	L 89x89x7.9	
REDUNDANT DIAGONAL	RD2	L 89x89x6.4	
	RD1	L 89x89x7.9	
REDUNDANT HIP	HIP	L 38x38x3.2	
INNER BRACE	IB	L 38x38x3.2	

**GENERAL NOTES:**

- FOR GENERAL NOTES, SEE SHEETS S-001 AND S-002.
- SCHEDULES APPLY TO TOWER SECTIONS AS SHOWN ON SHEETS S-300 THRU S-304.

PROTOTYPE  
DESIGN  
SUBMITTAL

SYMB	DESCRIPTION	DATE	APP
A	PROTOTYPE DESIGN SUBMITTAL	08/15/10	SCW

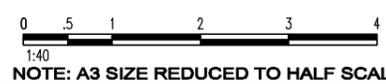
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MEMBER SCHEDULE

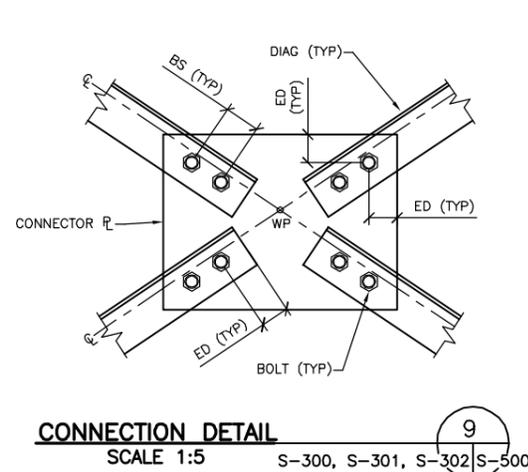
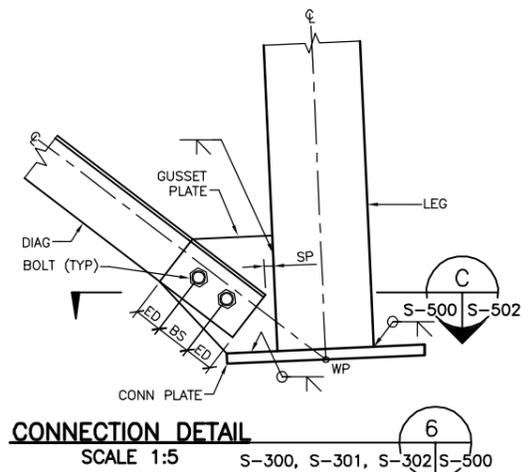
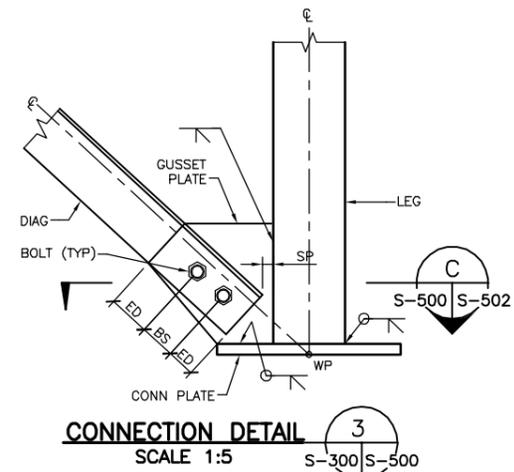
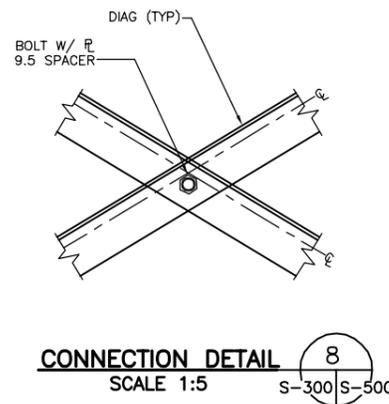
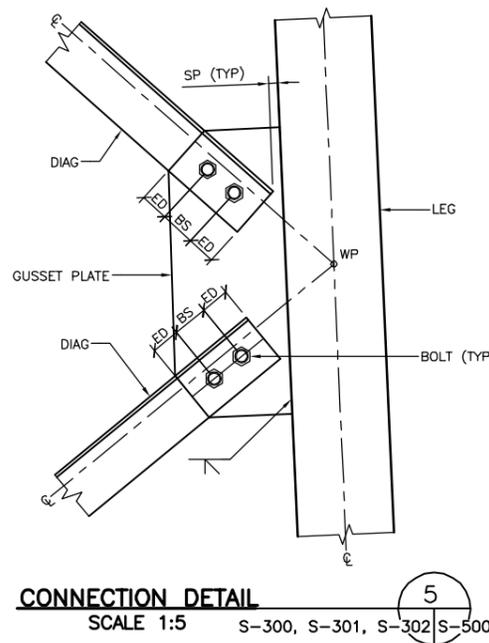
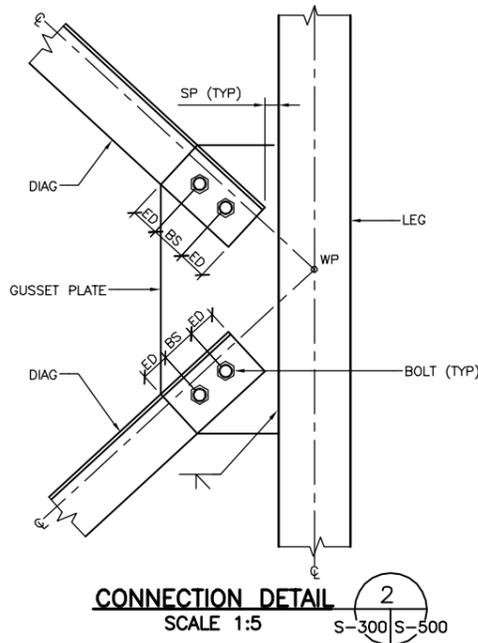
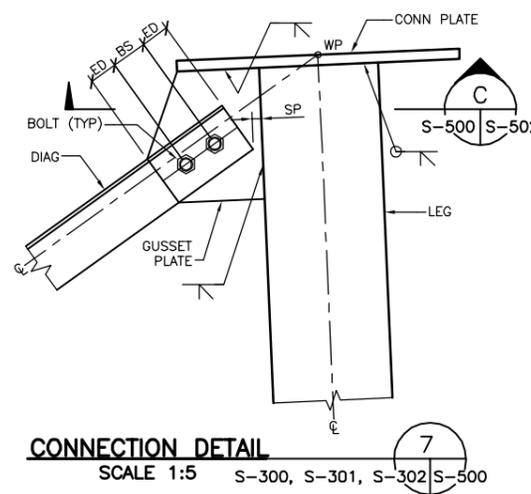
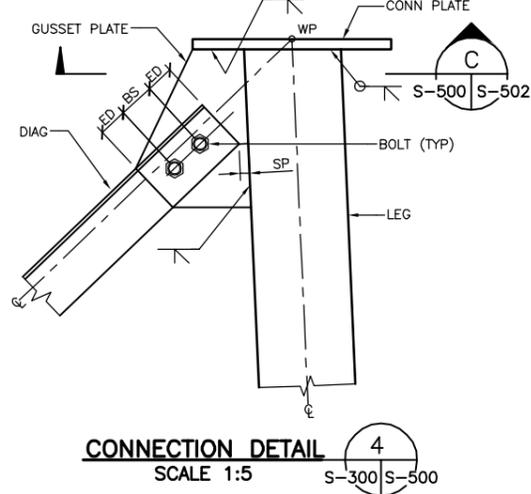
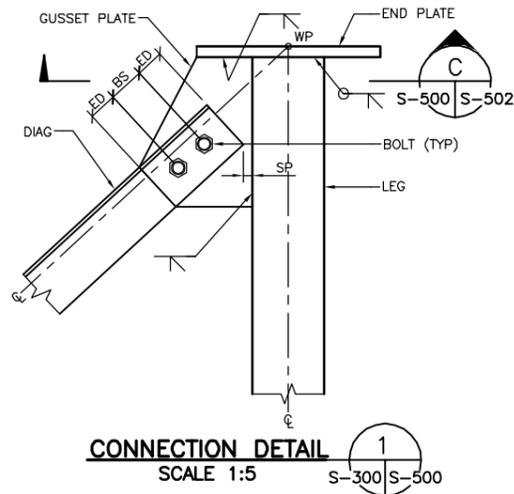
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**TWR00  
S-305**

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**GENERAL NOTES:**

- FOR GENERAL NOTES, SEE SHEETS S-001 AND S-002.
- CONTRACTOR TO SUBMIT GUSSET DETAILS FOR EACH JOINT BASED ON ITS SPECIFIC GEOMETRY AND THE FOLLOWING:  
CLEAR SPACE (SP) = 13 MIN.  
END DISTANCE (ED) = 40 MIN.  
BOLT SPACING (BS) = 50 MIN.
- "ED" SHALL BE MAINTAINED IN ALL DIRECTIONS AROUND BOLTS ON GUSSET PLATES.
- GUSSET WIDTH SHALL BE 150 MIN.
- FOR LEG AND BRACING SIZES SEE SECTION SCHEDULES.
- GUSSET PLATE THICKNESS:  
SECTION T1-T9: 9.5  
SECTION T10-T13: 12.7

PROTOTYPE  
DESIGN  
SUBMITTAL

SYMB	DESCRIPTION	DATE	APP
A	PROTOTYPE DESIGN SUBMITTAL	08/15/10	SCW

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FPH		

USAID THREE TOWERS  
PROTOTYPE DESIGN, AFGHANISTAN  
DIAGONAL CONNECTION DETAILS  
SHEET 1 OF 2

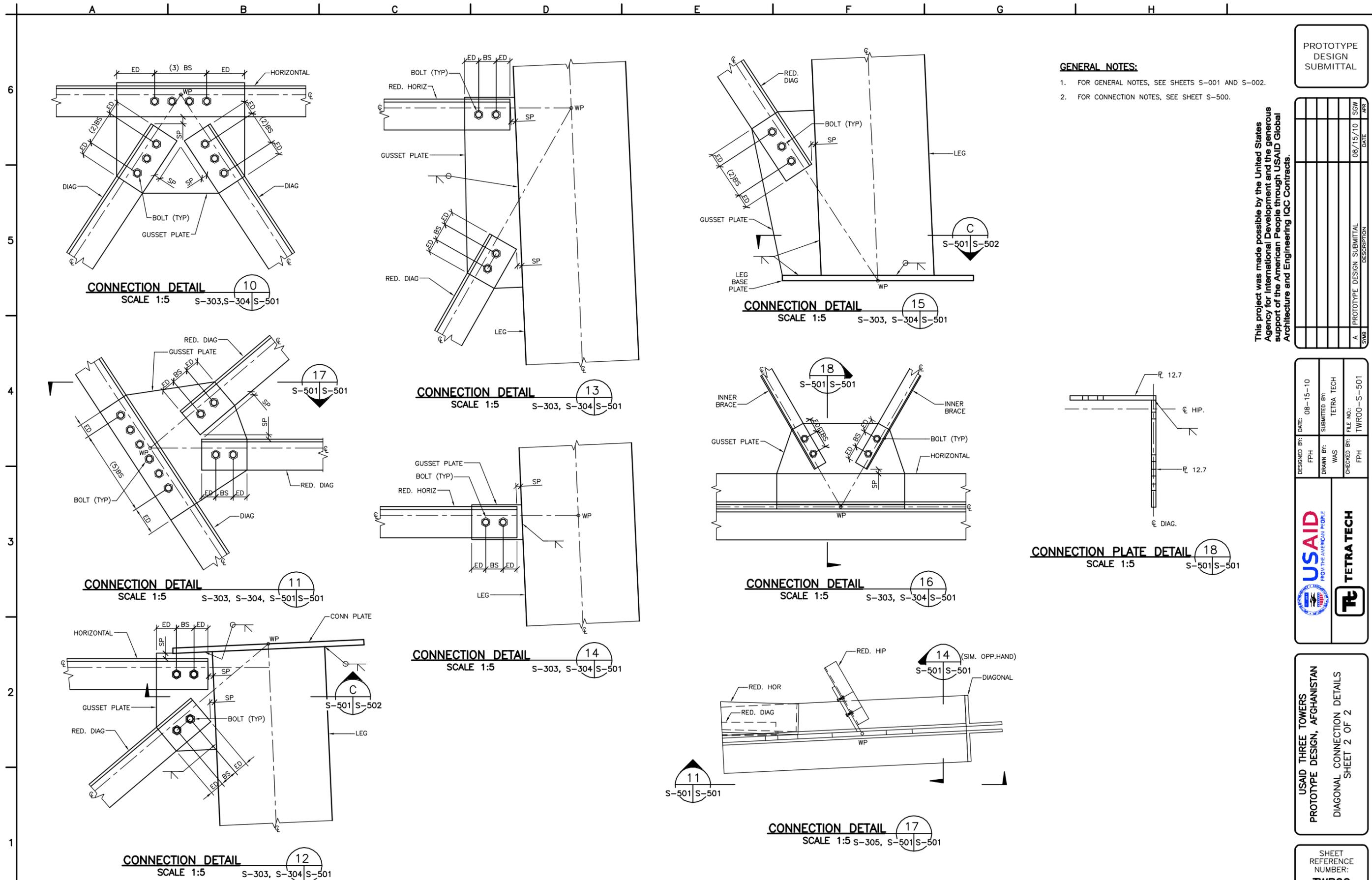
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NUMBER:  
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S-500**

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- GENERAL NOTES:**
- FOR GENERAL NOTES, SEE SHEETS S-001 AND S-002.
  - FOR CONNECTION NOTES, SEE SHEET S-500.

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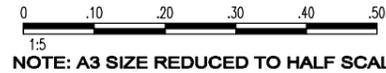
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SYMB	DESCRIPTION	DATE	SCW	APP
A	PROTOTYPE DESIGN SUBMITTAL	08/15/10		

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USAID THREE TOWERS  
 PROTOTYPE DESIGN, AFGHANISTAN  
 DIAGONAL CONNECTION DETAILS  
 SHEET 2 OF 2

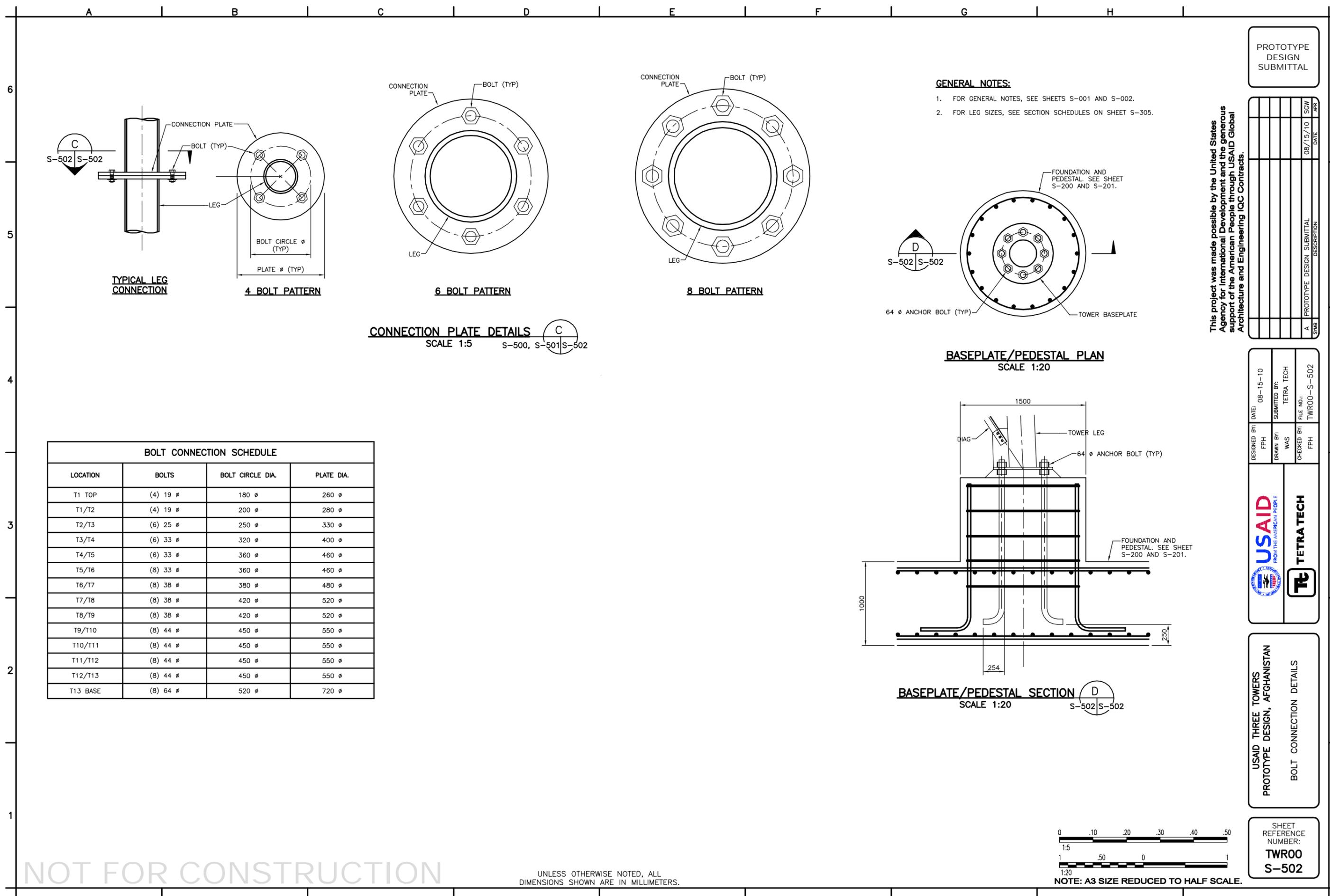
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**TWR00 S-501**



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BOLT CONNECTION SCHEDULE			
LOCATION	BOLTS	BOLT CIRCLE DIA.	PLATE DIA.
T1 TOP	(4) 19 $\phi$	180 $\phi$	260 $\phi$
T1/T2	(4) 19 $\phi$	200 $\phi$	280 $\phi$
T2/T3	(6) 25 $\phi$	250 $\phi$	330 $\phi$
T3/T4	(6) 33 $\phi$	320 $\phi$	400 $\phi$
T4/T5	(6) 33 $\phi$	360 $\phi$	460 $\phi$
T5/T6	(8) 33 $\phi$	360 $\phi$	460 $\phi$
T6/T7	(8) 38 $\phi$	380 $\phi$	480 $\phi$
T7/T8	(8) 38 $\phi$	420 $\phi$	520 $\phi$
T8/T9	(8) 38 $\phi$	420 $\phi$	520 $\phi$
T9/T10	(8) 44 $\phi$	450 $\phi$	550 $\phi$
T10/T11	(8) 44 $\phi$	450 $\phi$	550 $\phi$
T11/T12	(8) 44 $\phi$	450 $\phi$	550 $\phi$
T12/T13	(8) 44 $\phi$	450 $\phi$	550 $\phi$
T13 BASE	(8) 64 $\phi$	520 $\phi$	720 $\phi$

PROTOTYPE DESIGN SUBMITTAL	
SYMB	DESCRIPTION
A	PROTOTYPE DESIGN SUBMITTAL
DATE	DATE
SCW	SCW
APR	APR

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CHECKED BY:	FPH	
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USAID THREE TOWERS  
PROTOTYPE DESIGN, AFGHANISTAN

**USAID**  
FROM THE AMERICAN PEOPLE

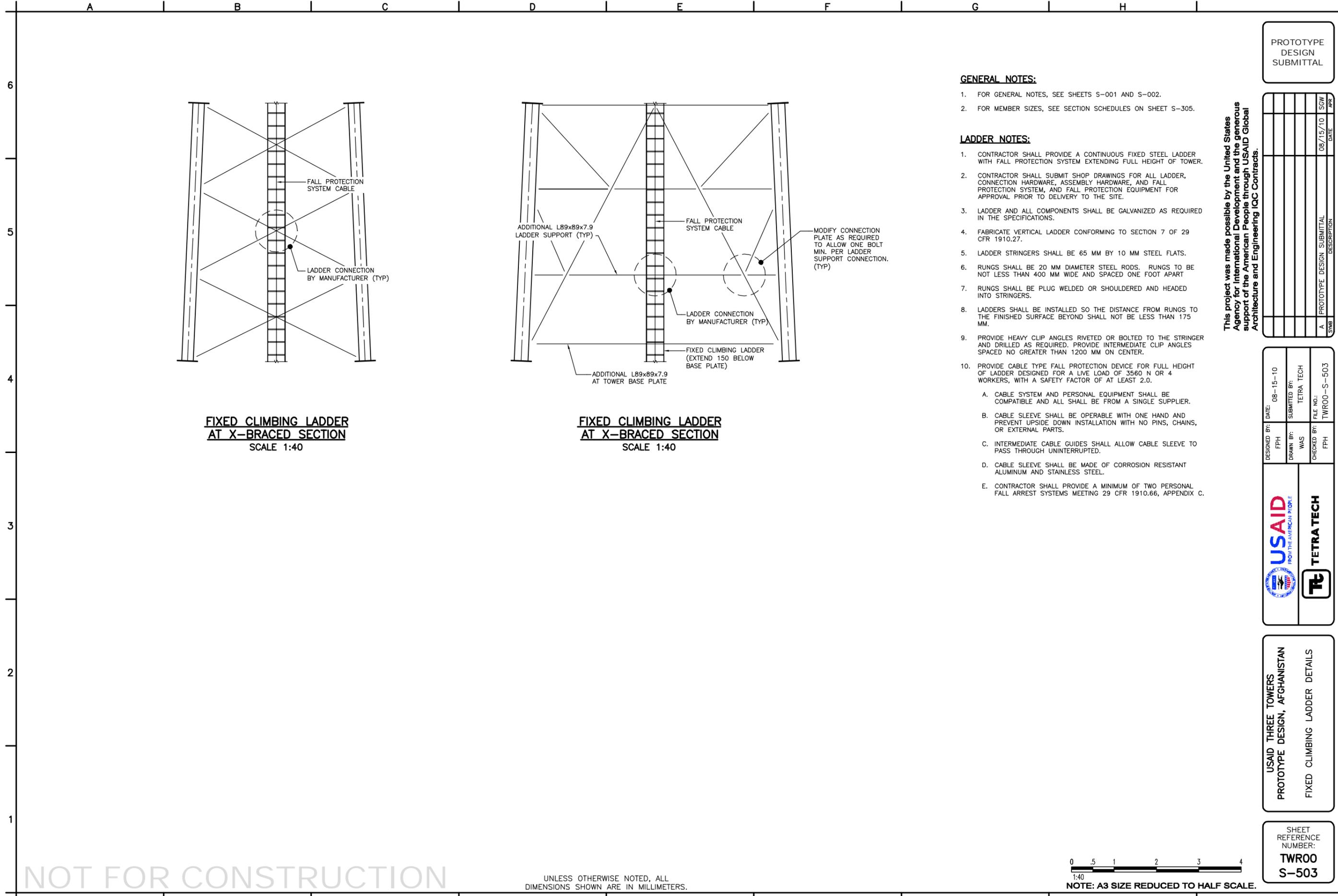
**TETRA TECH**

BOLT CONNECTION DETAILS

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**TWR00 S-502**

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**GENERAL NOTES:**

- FOR GENERAL NOTES, SEE SHEETS S-001 AND S-002.
- FOR MEMBER SIZES, SEE SECTION SCHEDULES ON SHEET S-305.

**LADDER NOTES:**

- CONTRACTOR SHALL PROVIDE A CONTINUOUS FIXED STEEL LADDER WITH FALL PROTECTION SYSTEM EXTENDING FULL HEIGHT OF TOWER.
- CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR ALL LADDER, CONNECTION HARDWARE, ASSEMBLY HARDWARE, AND FALL PROTECTION SYSTEM, AND FALL PROTECTION EQUIPMENT FOR APPROVAL PRIOR TO DELIVERY TO THE SITE.
- LADDER AND ALL COMPONENTS SHALL BE GALVANIZED AS REQUIRED IN THE SPECIFICATIONS.
- FABRICATE VERTICAL LADDER CONFORMING TO SECTION 7 OF 29 CFR 1910.27.
- LADDER STRINGERS SHALL BE 65 MM BY 10 MM STEEL FLATS.
- RUNGS SHALL BE 20 MM DIAMETER STEEL RODS. RUNGS TO BE NOT LESS THAN 400 MM WIDE AND SPACED ONE FOOT APART
- RUNGS SHALL BE PLUG WELDED OR SHOULDERED AND HEADED INTO STRINGERS.
- LADDERS SHALL BE INSTALLED SO THE DISTANCE FROM RUNGS TO THE FINISHED SURFACE BEYOND SHALL NOT BE LESS THAN 175 MM.
- PROVIDE HEAVY CLIP ANGLES RIVETED OR BOLTED TO THE STRINGER AND DRILLED AS REQUIRED. PROVIDE INTERMEDIATE CLIP ANGLES SPACED NO GREATER THAN 1200 MM ON CENTER.
- PROVIDE CABLE TYPE FALL PROTECTION DEVICE FOR FULL HEIGHT OF LADDER DESIGNED FOR A LIVE LOAD OF 3560 N OR 4 WORKERS, WITH A SAFETY FACTOR OF AT LEAST 2.0.
  - CABLE SYSTEM AND PERSONAL EQUIPMENT SHALL BE COMPATIBLE AND ALL SHALL BE FROM A SINGLE SUPPLIER.
  - CABLE SLEEVE SHALL BE OPERABLE WITH ONE HAND AND PREVENT UPSIDE DOWN INSTALLATION WITH NO PINS, CHAINS, OR EXTERNAL PARTS.
  - INTERMEDIATE CABLE GUIDES SHALL ALLOW CABLE SLEEVE TO PASS THROUGH UNINTERRUPTED.
  - CABLE SLEEVE SHALL BE MADE OF CORROSION RESISTANT ALUMINUM AND STAINLESS STEEL.
  - CONTRACTOR SHALL PROVIDE A MINIMUM OF TWO PERSONAL FALL ARREST SYSTEMS MEETING 29 CFR 1910.66, APPENDIX C.

PROTOTYPE DESIGN SUBMITTAL

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USAID THREE TOWERS  
PROTOTYPE DESIGN, AFGHANISTAN  
FIXED CLIMBING LADDER DETAILS

SHEET REFERENCE NUMBER:  
**TWR00 S-503**

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NOTE: A3 SIZE REDUCED TO HALF SCALE.