





FINAL REPORT

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ASLP Project No. RU10-009

Aquatic Sciences L.P. 40 Centre Drive Orchard Park, NY 14127 Tel 1 716 667 3507 Fax 1 716 667 3509 Leakage Detection Survey of the Qaraoun Dam, Lebanon using Remotely Operated Vehicle



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Executive Summary

In April of 2010 a remotely operated vehicle (ROV) inspection was completed on the upstream face of the Qaraoun Dam in Lebanon. The survey was completed under contract with International Resources Group (IRG) of Washington, D.C. with funding from the United States Agency for International Development (USAID) for the Litani River Authority (LRA) who operates the facility. This inspection was for the purpose of identifying leaks between the joints on the face slabs of the dam. To complete the inspection, Aquatic Sciences L.P. of Orchard Park, New York, operated their *Seabotix LBV300XL* ROV equipped with a dye injection system along each joint on the dam face.

While inspecting the dam face general conditions of the concrete slabs were observed. In many areas offsets and misalignments of the slabs were noted. In some areas broken or spalled edges were identified. Silt accumulation at the deep sections of the dam was heavy, thus covering some features on the dam face.

Two areas with large leaks were identified. The first major leak was located at Vertical Joint 24 at 831.3 m elevation. This leak was on a length of the vertical joint below the third horizontal joint. The second area with a large volume leak was detected at Vertical Joint O1 just above the abutment at elevation 847.4 m. It appeared two leaks in the same proximity were identified here. This area had been reported by LRA as repaired in August of 2009. Other smaller leaks were identified at Joint 32 elevation 838 m, Joint 31 at 835 m, Joint 29 at 842 m and Joint O2 at 835 m.

Since the ROV inspection was completed in a timely manner, some additional time was used to inspect the power tunnel inlet, the irrigation tunnels inlet and the spillway structure. During the inspection, the ROV was able to positively identify problems along the face of the dam relating to the source of leaks. The goals of the inspection were completed successfully.

الملخص التنفيذى

في نيسان من العام ٢٠١٠، انتهت عملية الكشف (مسح التسرب) على الواجهة الامامية المحاكية للمياه في سد القرعون، بواسطة جهاز غاطس يمكن التحكم به من خارج المياه، حيث تم تنفيذ هذا المسح من قبل شركة Aquatic Sciences L.P. of Orchard Park, New York بموجب عقد مع شركة مجموعة الموارد الدولية (IRG) و بتمويل من الوكالة الاميريكية للتنمية الدولية (USAID) عبر برنامج دعم إدارة حوض نهر الليطاني (LRBMS) لصالح المصلحة الوطنية لنهر الليطاني التي تدير منشآت القرعون. ان الهدف من هذا المسح كان تحديد أماكن التسرب من الـ water stop أو الفواصل الكاوتشوكية التي تمنع دخول المياه إلى جسم السد من البلاطات الخرسانية التي تشكل الواجهة الأمامية المحاكية للمياه و لإنجاز المهمة وتحقيق الهدف قامت شركة Sciences يمت و لإنجاز المهمة وتحقيق الهدف قامت شركة Sciences يمت و محمه.

بشكل عام استطعنا من خلال هذا المسح ملاحظة نمو بعض الكائنات والطحالب بشكل غير منتظم على الواجهة المذكورة وتكسير في زوايا البلاطات التي تتشكل منها الواجهة. كذلك تم ملاحظة بعض الطمي في الأماكن العميقة من السد.

إن أهم النتائج التي افضى إليها هذا المسح هي اكتشاف نقطتان للتسرب يمكن اعتبار ها كبيرة وهي على الفاصل العامودي رقم ٢٤ وهي على ارتفاع ٨٤٧,٣ ASL م، أما نقطة التسرب الكبيرة الثانية تم اكتشافها على الفاصل العامودي رقم ٢٠ وعلى ارتفاع ٨٤٧,٤ م وفي الواقع هي عبارة عن نقطتين للتسرب ولكنهما متقاربتان لذلك تم اعتبار هما نقطة واحدة وهذه المنطقة أي منطقة التسرب المذكورة قد تم التويه عنها في تقارير المصلحة الوطنية لنهر الليطاني وخاصةً في تقرير آب ٢٠٠٩، كذلك تم تحديد نقاط تسرب صغيرة كثيرة، لاسيما على الفاصل ٢٢ عامودي وعلى ارتفاع ٢٠٩٨ م وعلى ارتفاع ٢٠٩٩ ارتفاع ASL معلى الفاصل ٣٦ عامودي وعلى ارتفاع ٨٣٨ م على الفاصل ٣٦ تسرب صغيرة كثيرة، لاسيما على الفاصل ٢٣ عامودي وعلى ارتفاع ASL معلم م على الفاصل ٣ ارتفاع ASL معلم معلى الفاصل ٢٣ عامودي وعلى ارتفاع ASL معمل م على الفاصل ٣ ومأخذ مشروع الري القناة ٥٠٩ كذلك منشأة التصريف الـ معمل إنتاج الطاقة في مركبا ومأخذ مشروع الري القناة ٥٩٠ كذلك منشأة التصريف الـ هوي الي هعمل إنتاج الطاقة في مركبا

لقد تمت المهمة بنجاح كبير، وتم تحديد أماكن التسرب بشكل دقيق، لذلك يمكن تقييم عمل الجهاز الغاطس ايجاباً.

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REPORT

Leakage Detection Survey of the Qaraoun Dam in Lebanon April 20-26, 2010

1.0 INTRODUCTION

International Resources Group (IRG), of Washington, D.C., contracted Aquatic Sciences L.P. (ASLP) of Orchard Park, New York to assist with the locating and assessment of leaks in the Qaraoun Dam concrete face located in Lebanon. The project was funded by the United States Agency for International Development (USAID) for the Litani River Authority (LRA). The dam is rock filled with a concrete face and is 60 m high, 1,100 m long. Created in 1959, it supplies three hydro electrical stations and provides irrigation water to both the Bekaa and the South regions. ASLP's purpose was to provide on-site inspection services using video equipment to show existing joint conditions along the face and at the bottom of the dam. Additionally, dye injection was undertaken to positively identify leakage through any cracks. The inspection was completed successfully between April 20, 2010 and April 26, 2010.

1.1 Facility Description

The Qaraoun Dam is located at the southern end of Lake Qaraoun, which is located in the southern region of the Bekaa Valley, and is Lebanon's largest man-made lake. The dam is 40 km southeast of Beirut and 30 km east of Saida at geographical location 33° 32.886' North, 35° 41.412' East. At 2 km wide, 5 km long, and 12 km² in surface area, this artificial lake was created by the dam in 1959. The lake is located on the Litani River. It starts in the north of Lebanon and rises in the Bekaa Valley, west of Baalbek and naturally empties into the Mediterranean Sea north of Tyre. The dam, which holds back Lebanon's longest river, is 60 m high and 1,100 m long. A gallery 6,400 m carries the water to the first underground hydroelectric station. The dam crest is at 861 m elevation and the maximum water level is at 858 m elevation.

The structure is over 160 m wide (upstream to downstream) at the base and 6 m wide at the top. The sealing surface of the rock filled dam is concrete slabs 40 cm thick. The concrete slabs are made in panels of varying dimensions; most panels are 14 m wide and 12 m high. There is approximately 35,000 m² of concrete surface.

The concrete panels are jointed by waterstops with voids above the waterstops filled with sealant. These joints were labeled at the time of the inspection along the length of the dam. The numbering method starts at the center abutment and carries outward. East of the abutment the first joints were labeled D, C, B, A. Beyond this, to the east, the joints were labeled from 1 to 48; Joints 3, 14 and 16 were omitted from the dam crest. To the west of the abutment the joints were labeled from O1 to O33. Horizontal joints were not labeled, so vertical measurements were related to elevation ranging from 800 m to 860 m above sea level.



The dam is operated and maintained by the Litani River Authority (LRA), which was formed in 1954 to manage the Litani River and Watershed water usage and hydro-electric power generation. Three hydro-electric power plants operate using the discharge of the dam.



Figure 1: Location of the Qaraoun Dam in Lebanon

2.0 METHODOLOGY

The inspection of the Qaraoun Dam required the use of a specialized remotely operated vehicle (ROV) outfitted with navigation sonar and additional purpose-built equipment.

2.1 ROV Equipment

Aquatic Sciences chose the *Seabotix LBV300XL* ROV due to its capabilities and size. The vehicle was ballasted to be neutrally buoyant in fresh water. The vehicle uses six electric thrusters to propel itself through the water. Four horizontal thrusters are for forward travel capable of pulling long tether lengths. The *Seabotix LBV300XL* has a vertical and a lateral thruster to enable the operator to move vertically and laterally through the water column. The vehicle was equipped with two variable intensity 50W Quartz-halogen lights to illuminate the area of inspection for the high resolution color camera. The ROV was also equipped with a second low-light wide-angle color camera.

The system utilizes 740 m of neutrally buoyant, high visibility umbilical cable. This umbilical houses both signal and power conductors (fiber optic and copper respectively), along with a Kevlar strength member and abrasion resistant protective jacket. The umbilical is neutrally buoyant in water to reduce the drag and allow for longer penetration distances. An ROV pilot controls the vehicle movement, lighting, and camera position from the surface with the use of a handheld control console.

The video signal is routed to the surface through a fiber optic cable in the umbilical; the fiber optic signal is converted in the reel junction box to an analogue video signal which is then fed into a high resolution video monitor for the pilot to view. The video signal is also recorded in real time in digital format. Audio commentary is added to document any points of interest and anomalies as they are seen during the inspection.

The *Seabotix LBV300XL* is approximately 40 cm long, 35 cm wide and 25 cm high weighing 15 kg. The size and weight of the *Seabotix LBV300XL* allows the vehicle to be deployed by hand to difficult access points. Appendix 5 contains a specification sheet for the *Seabotix LBV300XL*.



Figure 2: Seabotix LBV300XL Prior to Deployment

2.2 Navigation Sonar

A *Tritech Micron DST* scanning sonar operating at 650 kHz was mounted on the ROV for navigation purposes. The sonar, mounted upright, scans on a horizontal plane generating a plan view image of the surrounding area using an acoustic beam 35 degrees tall and 3 degrees wide.



Figure 3: Tritech Micron DST Sonar Beam Pattern

The sonar is positioned so as to provide a full 360 degree view around the ROV. Targets are identified by strong returns (yellow) and shadows (black).



Figure 4: Navigation Sonar

2.3 Additional Equipment

To aid in the detection of leaks along the joints on the dam face bright nylon cords were attached to the front of the ROV to form "telltales". Upon the presence of flow, the strings are pulled toward the source of the leak, providing a quick visual representation in front of the video camera.

As a further indicator of flow, a dye injection system was integrated onto the ROV. This system uses a bladder of premixed water soluble dye tablets piped to a mini pump coupled to the ROV actuator. The pilot is able to meter out quantities of dye on demand, useful for monitoring for leaks with the color camera.

2.4 Inspection Process

The ROV equipment was mobilized via air freight to Beirut, Lebanon. An open bed truck was used to transfer equipment form the shipping depot to the dam site. Warehouse facilities were provided to help with the mobilization and security of the ROV equipment. The system was setup in the rear of a work vehicle to provide shelter for the equipment and operator from weather and sunlight. The ROV tether and winch were positioned at the back of the work vehicle and were accessible to the tender during operations. The client supplied a 6000 watt generator which was the source of electrical power for all inspections.

During the inspection the support vehicles were positioned near the area being surveyed. The ROV was lowered by its tether down the face of the dam to the water. The ROV then carried out the inspection controlled by the operator, with the tether managed by the tender at the dam crest. Throughout the inspection audio commentary was added to the video detailing conditions. As well, the client provided a drawing that was marked digitally with the progress of the inspection and any faults or areas of interest.





Figure 5: ROV being lowered down the dam face to the water

The inspection initially originated at the west end of the dam and traveled east. On the first day the joints near the water surface were prioritized since water levels were expected to lower over the inspection period. After completing the upper portion of the dam, the inspection proceeded along deeper joints. Due to changing weather conditions the survey shifted from area to area to keep within the schedule. On April 25, 2010 an outage of the Markabi power house was scheduled between 06:00 and 10:00 for inspection of the tunnel inlet and surrounding dam face.

In general, each vertical joint was inspected from top to bottom. On the ascent, horizontal joints were inspected leading off the vertical joint. The ROV then returned to the vertical joint and continued ascending. This approach was required to reduce the risk of entanglement on the artifacts along the dam face.

Following the successful completion of the entire dam face, further inspections were requested by LRA. These inspections took place at one of the irrigation intakes checking for sediment and the overflow structure checking for leaks. For both of the additional inspections a boat was used to aid in tending the ROV away from the dam. Finally, on April 26, 2010 the ROV was used to inject dye into one of the leaks found on the dam face.

Following the additional inspection work, the ROV was demobilized and packed into shipping crates. The crates were then returned to the shipping depot by an open bed truck for return transit. A brief meeting was held with IRG and LRA to discuss the findings and proposed solutions. Additional site photographs are provided in Appendix 4.



3.0 INSPECTION OBSERVATIONS

During the period from April 20, 2010 to April 26, 2010, a remotely operated vehicle (ROV) inspection was completed on the face of the Qaraoun Dam in Lebanon. During the time of the inspection the water level was high, between 856.30 m and 856.84 m. Weather varied over the period from mostly sunny to thunderstorms with rain. Aquatic Sciences L.P. of Orchard Park, New York completed the inspection under contract by International Resources Group (IRG) funded by the United States Agency for International Development (USAID). The Qaraoun Dam is managed and maintained by the Litani River Authority (LRA).

The inspection of the dam started at the west end working east along the upper section. The lower sections of the dam were completed in changing order depending on area. Documentation of the joint conditions were made during the inspection. This information is provided in Appendix 2. All references were labeled with the date and time matching the video and sonar data. Video still images are available in Appendix 1.

3.1 Limitations

The inspection completed by Aquatic Sciences L.P. in April 2010 was intended for the purposes of locating leaks, i.e., identified water flow from upstream to downstream, on the face of Qaraoun Dam. The inspection is based only on visible features/areas of the dam below water on the dates of the inspection. This inspection did not entail structural, stability or hydraulic investigations. General conditions of the concrete panels, their joints and features were noted in video commentary and this report, however are in no way inclusive of all characteristics present. The report does not include the evaluation from a specialist on the general condition, safety or stability of the structure.

3.2 General Findings

During the inspection of the Qaraoun Dam, water elevations were recorded daily from the water level staff mounted on the southwest side of the overflow structure. The water level dropped uniformly over the inspection period from 856.83 m to 856.30 m. Water elevations are used with reference to the ROV depth gauge to determine vertical position on the dam face.

| Date | Elevation (m) |
|------------|---------------|
| 2010-04-20 | 856.84 |
| 2010-04-21 | 856.74 |
| 2010-04-22 | 856.62 |
| 2010-04-23 | 856.55 |
| 2010-04-24 | 856.46 |
| 2010-04-25 | 856.38 |
| 2010-04-26 | 856.30 |

Table 1: Water elevations during the inspection period

Reporting for the Qaraoun Dam has been divided into seven sections from left to right as viewing from upstream. In general, the conditions were observed to be similar across the face of the dam. Joints were variable in alignment and spacing. In most cases horizontal joints were small and well aligned, vertical joints were larger in separation. The inspection observations map in Appendix 2 provide locations to features documented during the inspection. The maps were drawn with information and measurements collected at the dam, as well as a drawings (dated December 12, 1979) provided by the client/owner.

3.2.1 Section 1

Refer to Appendix 2-1.

Observation Map Sheet 1 covers from vertical mark 49 to 44. The location marked 49 is the end of the dam structure at the east. Between joint marks 49 and 47 no water was present at the base of the slabs at the time of inspection. The ROV had adequate water depth to work from Joint 46 and west. This area was inspected on April 22, 2010. At the junction of Vertical Joint 45 and the upper horizontal joint some spalling on the slab edge was noted. The bottom had little debris and silt accumulation. No leaks were detected in this area.

3.2.2 Section 2

Refer to Appendix 2-2.

Observation Map Sheet 2 covers from Vertical Joint 43 through to Vertical Joint 30. A number of areas were noted with raised offset slab sections. Along the upper horizontal joint between Vertical Joint 43 and 44, at Vertical Joint 38 and at Vertical Joint 32 the greatest offsets were noted. In these areas the upper slabs were lifted.

Some broken concrete edges were noted in this area on the upper horizontal joint between Vertical Joint 37 and 38 and along Vertical Joints 34 and 36. A minimal amount of silt and debris was on the bottom in this area.

Three small leaks were noted in this section; one along Vertical Joint 32 between elevations 837 m and 842 m and the second small leak at elevation 835 m along Vertical Joint 31. The third leak was at elevation 836.1 m along Vertical Joint 33.

3.2.3 Section 3

Refer to Appendix 2-3.

Sheet 3 covers the area from Joint 29 through to Joint 15. At Vertical Joints 27 and 24 offsets were noted at the top of the dam above the water line. Along the upper horizontal joint in this section the upper slabs were generally raised. Numerous areas with shifts and offsets were noted.

Two leaks were identified in this section; one along Joint 29 at elevation 840.1. The second, a large leak at 831.3 m elevation along Joint 24. This second leak was recorded on April 22 at 16:16 and was observed to be less than 1 m in length with an opening 2 cm wide. A high velocity of flow was noted through this opening, evident by the clean concrete surrounding the leak.

3.2.4 Section 4

Refer to Appendix 2-4.

In Sheet 4, the deepest section of the dam is covered from Vertical Joint 13 to Vertical Joint D. The dam appeared to have few offsets and misaligned joints in this area. Across the upper horizontal joint at 851 m a continuous offset where the upper panels were raised above the second row of panels was observed. Near the area east of the abutment at Vertical Joint C, larger offsets were noted where the east panels were lower than those adjacent to the abutment.

In a few areas spalling was noted on joint edges including: Joint 13 at 842 m, Joint 6 at 851 m and Joint 2 below 851 m. In the deeper section of the dam several more rows of scaffold bracketing were observed below each horizontal joint. In some areas ladders were present along the vertical joints. Rope, netting and other debris were attached or present on and around the scaffold sections, shown in Image 1-9 in Appendix 1. On the bottom a thick layer of silt was present generally obstructing a view of the horizontal joints. A variety of natural and artificial debris was present on the reservoir floor.

Along Vertical Joint C and D, both at 855 m elevation a possible leak was observed.

3.2.5 Section 5

Refer to Appendix 2-5.

Sheet 5 covers the area heading west of the abutment from Joints O1 to O16. The vertical joints near the abutment here are raised on the east side, as the west panels move backwards. Along Joint O2 the offset at 840 m was measured at approximately 25 cm. Some other offsets were noted along Joint O5 and at the upper horizontal joint at O9 and O16. In many areas the edges of the concrete slabs were broken.

A large leak was observed at Vertical Joint O1 on April 25 at 14:35. This leak appeared to be present in two locations near the third horizontal joint down. This was an area that had been repaired in August of 2009. Another possible leak was identified just above this large leak on Joint O2 and below at elevation 834.8 m. Some other possible leaks were noted at Joint O8 at 854.6 m and at Joint O10 at elevation 855.7 m.

3.2.6 Section 6

Refer to Appendix 2-6.

In Sheet 6, the area spanning from Vertical Joint O17 to O30 is covered. In general, most offsets were observed along the interface between the upper two rows of slabs where the upper slab is usually raised outwards. Joints O18 and O19 had broken edges along the vertical joints. At Joint O24 and elevation 842 m the slabs were offset with spalled edges.

No leaks were identified in this area.

3.2.7 Section 7

Refer to Appendix 2-7.

Sheet 7 provides coverage of the west end of the dam, from Vertical Joint O31 to O33. This area was shallow with few features noted. No leaks were observed in this section.

3.3 Identified Leaks

At the time of the April 2010 ROV inspection of the Qaraoun Dam, two major leaks and several smaller leaks were identified.

The first major leak was located at Vertical Joint 24 at 831.3 m elevation. This leak was on the vertical joint below the third horizontal joint. The leak had a high volume of flow through an opening 2 cm wide and less than 1 m high, found on April 22 at 16:16. (See Image 1-6 & 1-7 in Appendix 1.)

The second area with a large volume leak was detected at Vertical Joint O1 just above the abutment at elevation 847.4 m. It appeared two leaks in the same proximity were identified here on April 25 at 14:09 and 14:35. This area had been reported by LRA as repaired on August 14, 2009. On April 26, 2010, 1.5 liters of dye was injected here to aid in locating the path of the leak (shown in Image 1-22, Appendix 1).

Other leaks were identified at Joint 32 elevation 838 m, Joint 31 at 835 m, Joint 29 at 842 m and Joint O2 at 835 m. These joints passed less volume than those mentioned above.

3.4 Additional Areas Inspected

In addition to the leak inspection of the dam structure, other locations were also inspected at the Qaraoun Dam. These included the intake for the power tunnel, the intake for the irrigation tunnels and the spillway structure.

3.4.1 Power Tunnel Intake

On April 25 at 07:45 during the scheduled outage, the ROV was used to inspect the opening of the power tunnel intake to observe the general condition and check for accumulation of debris. The intake was located using navigation sonar and inspected while the ROV was tended from the dam face.

Using sonar, the intake opening was measured at 3 m wide. The depth gauge was used to measure the open clearance in elevation of the intake as 4.6 m. Twelve horizontal vanes were counted equally spaced along the openings height. All horizontal surfaces had some silt accumulation present (Image 1-13, Appendix 1). The structure appeared to be in good condition with no signs of deterioration or damage. Additional details are present in Figure 3-2, Appendix 3 in the collected sonar imagery.

3.4.2 Drainage Intakes

Two drainage intakes are present north of Vertical Joint 19 approximately 100 m from the dam. The ROV navigation sonar was used to locate these structures with the ROV from in front of the dam. In order to limit the risk of entanglement, only the west intake structure was observed with the ROV.

The structure was approached from the west side and descended to the top of the inlet. The concrete appeared to be in good condition. Very little silt and debris was present on the structure. The top of the intake was several meters above the silt layer on the reservoir floor.

Using sonar, the structures appeared to be hexagonal in shape with features running south towards the dam. Appendix Figure 3-4 shows three I-beam lengths between the west intake and the dam face. The east intake was also observed by using the navigation sonar.

3.4.3 Spillway Structure

The reservoir spillway structure is north of Vertical Joint 23 about 50 m from the dam. An inspection of this structure was requested by LRA for the locating of leaks. The ROV was tended from the structure while operated from the dam. The south side was inspected, where a notable number of patches of rough concrete forming were observed. The underside edge of the overflow flume was rough with small holes present (Image 1-18, Appendix 1). No leaks were positively identified by the ROV.

The inside of the spillway risers were observed from the top of the structure. Several small leaks were visible. Photographs of the inside of the spillway are provided in Appendix 4.

4.0 SUMMARY

From April 20, 2010 to April 26, 2010, a remotely operated vehicle (ROV) inspection was completed on the upstream face of the Qaraoun Dam in Lebanon. This inspection was for the purpose of identifying leaks between the joints on the face slabs of the dam. To complete the inspection, Aquatic Sciences L.P. of Orchard Park, New York, operated their *Seabotix LBV300XL* ROV equipped with a dye injection system along each joint on the dam face.

While inspecting the dam face, general conditions of the concrete slabs were observed. In many areas offsets and misalignments of the slabs were noted. In some areas broken or spalled edges were identified. Silt accumulation at the deep sections of the dam was heavy covering some features on the dam face.

Two areas with large leaks were identified. The first major leak was located at Vertical Joint 24 at 831.3 m elevation. This leak was on the vertical joint below the third horizontal joint. The second area with a large volume leak was detected at Vertical Joint O1 just above the abutment at elevation 847.4 m. It appeared two leaks in the same proximity were identified here. This area had been reported by LRA as repaired in August of 2009.

Other smaller leaks were identified at Joint 32 elevation 838 m, Joint 31 at 835 m, Joint 29 at 842 m and Joint O2 at 835 m.

Since the ROV inspection was completed in a timely manner, additional time was used to inspect the power tunnel inlet, the irrigation tunnels inlet and the spillway structure.

During the period of the inspection the ROV was able to positively identify problems in the face of the dam relating to the source of leaks.

APPENDIX 1:

Video Images



Image 1-1: 2010-04-20-1703 - Joint offset at joint O9 and upper horizontal joint.



Image 1-2: 2010-04-21-0854 - Joint intersection at joint O6 and upper horizontal joint.



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Image 1-3: 2010-04-21-1440 - Vertical joint 25.



Image 1-4: 2010-04-21-1737 - Leak in vertical joint 29.



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Image 1-5: 2010-04-22-0955 - Ladder at vertical joint 21.



Image 1-6: 2010-04-22-1617 - Large leak closeup at joint 24.



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Image 1-7: 2010-04-22-1618 - Large leak in vertical joint 24 at 831.3 m elevation.



Image 1-8: 2010-04-24-1545 - Raised slab at joint O2.



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Image 1-9: 2010-04-24-1706 - Net, ropes and debris hanging on platform near joint 10 at elevation 815 m.



Image 1-10: 2010-04-25-0635 - Debris at bottom of abutment.



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Image 1-11: 2010-04-25-0635 - Raised slab along joint C.



Image 1-12: 2010-04-25-0731 - Silt at floor of power tunnel inlet.



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Image 1-13: 2010-04-25-0734 - Horizontal vanes in power tunnel inlet.



Image 1-14: 2010-04-25-0746 - Silt on vane in inlet.



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Image 1-15: 2010-04-25-0903 - Broken edges of slab at joint 2.



Image 1-16: 2010-04-25-1436 - Dye being pulled in to large leak in joint O1



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Image 1-17: 2010-04-25-1436 - Leak at joint O1.



Image 1-18: 2010-04-26-1052 - Porous concrete interface on spillway.



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Image 1-19: 2010-04-26-1209 - Irrigation intake structure.



Image 1-20: 2010-04-26-1209 - Top of irrigation intake.



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Image 1-21: 2010-04-26-1214 - I-Beams leading south from intake.



Image 1-22: 2010-04-26-1254 -Injecting dye into large leaks at O1.



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APPENDIX 2:

Inspection Observations Maps



| SHEET 01 OF 07 | SHEET 02 OF 07 | SHEET 03 OF 07 | SHEET 04 OF 07 | SHEET 05 OF 07 | SHEET 06 OF 07 | SHEET 07 OF 07 |
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| | | | | | | AVERAGE T WATER ELE (20 THROL | OP OF VATION - 856.60M | | | | |
|--------------------------------------|-----------------------|---|------------------------|---------------------------------------|----------------|-------------------------------------|------------------------------|------------------|----------------------------------|-------------------------|--------------------|
| 1 | 13 | 12 | 1 1 | 0 | 9 8 | | 7 | 6 | 5 | 4 | 2 |
| B61.0 | (21-1225 N) | (21.1219 NI | (21-1210) | (21-1202 NII)- | (21-1157 NI) | (21, 1150 M | | | (21-1121 NI)- | | |
| (21-1232 NL) | | | | (21-1202 NL) | | (21-1130 NE) | | | | (21-111-4 NL) | |
| (21-1228 NL) | (21-1227 NL) | 21-1214 NL RS UPPERS | 21-1213 NL | (21-1200 NL) | (21-1155 NL)- | (21.1149 NL) | 21-1140 NL BROKEN CORNERS | (21·1138 NL) | (21.1132 NL) | (21-1127 NL) | |
| 23-1320 NL | (21-1224 NL)-/ | (23·1457 NL) | (24-1700 NL) | 21.1206 NL SPALLING EDGE | (25-1216 NL) | | | RS UPPERS 4CM | 0FFSET | 21-1121 NL RS UPPERS | 21 RS 21 |
| · | 23-1413 NL)7 | (23-1451 NL)7 | | (24-1727 NL)7 | | (25-1204 NL)7 | (25-1136 NL)7 | (25-1020 NL)7 | (25-0950 NL) | (25-0925 NL)7 | REC FRA (25) |
| (23-1258 NL) | 842.0 (23-1442 NL) | | | | | | | | | | (25-0 |
| 3-1410 NL PALLED CORNERS | (23-1334 NL)7 | | | (24·1724 NL)-7 | (25-1234 NL)-7 | (25-1201 NL)- | (25-1133 NL) | | 25-0947 NL SC | | 25-0 SC |
| 834.0 | | | (24-1711 NL) | | | | | | | | |
| 23-1324 NL PLATFORM | | | | | | | (25-1131 NL) | (25-1017 NL) | | | |
| 826.0 | | | | | 25-1232 NL | | | | | - | |
| | | 23-1504 NL PLATFORM PARTIALLY DISMANTLED | - - | | SMALL PLATFORM | | | (75 1010 H) | 25-0945 NL LADDER ALONG JOINT | 25-0913 NL | |
| 23-1251 NL | 818.0 | | 24-1705 NL PLATFORM | (25-1330 SC) | 25-1230 NL | | (25-1129 NL) SC | (23.1012 NL) | | | |
| 813.0 | | 22.1427 NI | | | | 25-1158 NL | | | | | |
| | | (23 1437 NL) | | 24-1710 HEAVY SILT IN THIS AREA | (25-1225 NL) | | | | | | |
| 26-1326 NL FISHING OR FALL NET | 808.0 | | | | | | 25-1124 NL HEAVY SILT | 25-1010 NL SC | | | |
| | | 804.0 | | | | | | | | | |
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| | | | | | | | | | | |
| | | | | AVERAG WATER (20 TH | E TOP OF ELEVATION = 856.60m ROUGH 25 APRIL 2010) | | LADDER DO | OWN TO | | |
| 0 | 17 0 | 18 0 |)19 | 020 0 | 21 0 | 22 0 | 23 0 | 24 (| 025 02 | 26 |
| 861.0 | (20-1238 NL) | (20-1225 NL) | (20-1218 NL)- | | 20-1159 NL WIDE OJ | | (20-1136 NL)- | (20-1124 NL) | (20-1108 NL) | 20-1103 |
| (20-1247 NL) (20-1252 NL)7 | 20-1238 NL RS UPPER 5cm | 20-1236 NL FILLER | 20-1222 NL FILLER | 20-1214 NL RS, J0 TOP RIGHT CORNER | 20-1202 NL FILLER | (20-1148 NL) (20-1155 NL) | 20-1145 NL | (24-0839 NL) (20-1128 NL) FILLER | (23-1734 NL) (23-1737 NL) | (23-1720) (23-1727 NL) |
| 851.0 | 24-1018 N | (24-1003 NL) | (24-0950 NL) | (20-1206 NL) | | 24-0911 NL JO UPPERS WEST | | | | |
| (24-1118 NL) | <u>BE</u> 24-1029 NL | (24-1013 NL) | 24-0957 NL | (24-0938 NL) (24-0944 NL) | (24-0925 NL) (24-0932 NL) | (24-0917 NL) | (24-0906 NL)7 | 24-0903 NL RS, BROKEN CORNERS | 24-0848 NL 845.0 | 23-1724 |
| 842.0 | 24-1024 NL SC | 24-1011 NL SC | 24-1006 NL 0J | | 24-0927 NL SC | 838.5 | | (24-0846 NL) | | |
| 837.5 | | | (24-0954 NL) | 24-0942 NL SC | | 00013 | | | | |
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| SHEET O1 OF O7 | SHEET 02 OF 07 | SHEET 03 OF 07 | SHEET 04 OF 07 | SHEET 05 OF 07 | SHEET 06 OF 07 | SHEET 07 OF 07 |
|----------------|----------------|----------------|----------------|----------------|----------------|----------------|



APPENDIX 3:

Sonar Images



Figure 3-1: 2010-04-25 07:32:20 - Power tunnel intake overview with rock cutaway



Figure 3-2: 2010-04-25 07:32:51 - Power tunnel intake closeup

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Figure 3-3: 2010-04-26 12:12:24 - Irrigation tunnel inlets, presumed hexagonal shape.







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APPENDIX 4:

Site Photographs



Image 4-1: ROV being lowered through surface debris.



Image 4-2: ROV near vent pipe.



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Image 4-3: System setup in rear of work vehicle.

Image 4-4: Spillway and offset at joint 24.

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Image 4-5: View looking west along dam face showing bend to south.

Image 4-6: Boat used to aid in recovering ROV tether from leak.

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Image 4-7: Drainage outlets with combined leaks in small conduit.

Image 4-8: Accessing the spillway structure.

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Image 4-9: View down small spillway riser.

Image 4-10: View looking down large spillway riser.

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APPENDIX 5:

Equipment Description

REMOTELY OPERATED VEHICLE **LBV300XL** INSPECTION SERVICES

Sciences provides Aquatic umbilicals of 250m (820 ft.),750 m (2,460 ft.), 1100 m (3,600 ft.), and 2,500m (8,200 ft.) for the LVB300XL, which is unique in the world for this class of inspection vehicle. This vehicle operates with increased provides thrust and forward superior quality video images. This Remotely Operated Vehicle (ROV) is approximately 53 cm (21 in.) long, 24 cm (10 in.) wide and will operate in a 41 cm (16 in.) diameter pipe.

LBV300XL is depth rated to 304 m (1,000 ft.) and weighs less than 15.8 kg (35 lbs.). This inspection class ROV features compact size, powerful thrusters, and maximum manoeuvrability.

The system provides on screen information overlay to aid in navigation, setup and control. Information displayed includes heading, depth, water temperature, camera angle, date/time, trim, light level, and user text.

40 Centre Drive Quaker Centre Business Park Orchard Park, NY 14127 716-667-3507 Tel 716-667-3509 Fax

www.aquaticsciences.com

CAMERAS & FUNCTIONS

The high resolution colour camera requires minimal light, has a 3.6 mm lens, auto iris, manual focus and is mounted on a rotating chassis to provide 270 degree field of vision. The camera itself has full tilt capabilities, allowing extreme close-ups of tunnel and pipeline surfaces. A secondary forward camera features low light colour format for use when water conditions are turbid. This camera is a Videology CCD camera with a wide dynamic range (64 db). Hi resolution (450 TVL), low light (0.3 lux), DC auto Iris. The wide-angle 2.8 – 6mm lens offers a 118-degree horizontal field of view. The ROV is fitted with compact sonar for navigation purposes. A single function manipulator is standard on the ROV.

All camera functions, array of LED and halogen variable intensity control lights and a single function manipulator are controlled from the surface. Approximate dimensions are 53 cm (21 in.) in length and 24 cm (10 in.) wide.

A C C E S S O R I E S

The ROV can be retrofitted to include profiling sonar, probe for metal thickness measurement, scrub brush and a manipulator.

T H R U S T E R S

Six brush-less DC thrusters: four forward, one vertical and one lateral. The vehicle features modular thruster configuration and has a high thrust to weight capability enabling extended excursion capacity.

UMBILICAL CABLE

The fiber optic umbilical cables ASI provides are 250m (820 ft.),750 m (2,460 ft.), 1100 m (3,600 ft.), and 2,500 m (8,200 ft.) in length and 8 mm (0.31 in.) in diameter with a Kevlar strength member and a buoyant floatation jacket which counters the negative buoyancy of the internal leads. An electronic sheave counter measures the distance of penetration.

C O N T R O L S

All thruster, camera, light and manipulator functions are controlled from the surface via joystick box. Topside control of all functions allows clients to participate in the inspection process, directing the ROV pilot to areas of concern and adding real time audio commentary to the video document.

MOBILIZATION & DEMOBILIZATION

For shoreside access inspections, the service is completely self sufficient and operates from a specially outfitted cube van complete with genset, spool ramps, communications equipment and heaters.

APPENDIX 6:

Inspection Video DVDs

The DVDs included with this report contain the complete video collection from the Qaraoun Dam inspection in April 2010 in Windows Media Video (.wmv) format. This media is not viewable in standard DVD set top players.

The Highlights DVD is playable in NTSC DVD set top players. As well, the Highlights disc contains a Windows Media Video (.wmv) version of the video and this report in Adobe Acrobat (.pdf) format.